

# Stormwater Pollution Prevention Plan

For

**Caroga Lake Arts Collective, Inc.**

At

1989 State Rte. 10

Caroga Lake, NY

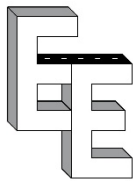
Prepared For:

Rick Ruby

P.O. Box 1048

Caroga Lake, NY 12032

Prepared By:



**Empire Engineering, PLLC**

1900 Duanesburg Road

Duanesburg, NY 12056

**June 23, 2021**

**Last Revised:**

**July 21, 2021**

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## Site Information & Evaluation

### Project/Site Information

The subject project is the commercial development of a parcel which currently has one existing building predominantly surrounded by trees with some brush. The subject site is located at the Northeast intersection of State Route 10 and County Route 112 in the Town of Caroga Lake, NY. The total parcel area is approximately 10.9 Ac with an initial project site of approximately 1.6 Ac. The property is identified by Tax Map numbers 68.17-2-1 & 68.17-2-2.

Anticipated Construction Start Date: September 2021

Anticipated Completion Date: November 2022 (Phase 1)

### Contact Information

#### Owner/Operator:

Caroga Lake Arts Collective, Inc.  
P.O. Box 1048  
Caroga Lake, NY 12032  
Contact: Rick Ruby (518) 844-2679

#### Engineer:

Empire Engineering, PLLC  
1900 Duanesburg Road  
Duanesburg, NY 12056  
Contact: Christopher Longo, PE (518) 280-1371

#### Contractor:

Owner/Operator

### Drainage Patterns & Topography

Runoff from the project site drains westerly through surface drainage to federally designated wetlands located along the western property boundary and an existing roadside swale along County Rte.112. Soils on the site are fine sandy loam as identified by the USGS soils classification and on-site test pits. The topography of the site can be generally described as sloped with a high point near the northeastern corner of the site. The site currently exists as a parcel with a vacant carriage house style structure with a small clearing and a paved drive. The site is predominately forested with some brush.

### Potential Sources of Pollution

The primary sources of pollution from an active construction site are erosion, siltation, debris transport, and accidental spills or leakage of oils from equipment.

### Implementation Schedule

The construction sequence outlined below should be followed or amended as necessary to minimize the susceptibility of the site to erosion and sediment transport during construction.

1. Establish perimeter protections and stabilized construction entrances within work area.
2. Construct temporary sediment traps in the location of permanent stormwater controls.

3. Once all erosion and sediment control measures are constructed and functional, disturbance may begin within that subject area.
4. Rough grade the project area, establish any swales and/or temporary check dams to divert runoff to storage areas.
5. Stabilize cut/fill slopes and stabilize internal roadway areas with subbase course as necessary.
6. Complete demolition, backfill and stabilize with suitable fill soils.
7. If the project is occurring in multiple phases repeat steps 1-4 in any new drainage area.
8. Upon completion of grading, final seeding and full vegetative cover shall be established.
9. Prior to finalizing connection to the storm sewer system, all catch basins and drainage lines shall be cleaned of all silt and sediment.
10. Once final stabilization is achieved remove all temporary erosion and sediment control measures including silt fence, storm structure protections and temporary sediment basin components.

### Notice of Intent

The owner shall submit a Notice of Intent (NOI) to the New York State Department of Environmental Conservation and obtain authorization of construction activities before commencing work. A copy of this NOI is included within Appendix A.

### Historic Preservation

The Office of Parks, Recreation & Historic Preservation database was reviewed for potential Historic or Cultural significant data at or near the project site. The database revealed that the site is not within an “archeologically sensitive area” and no further review is warranted. The database results are included within Appendix B.

### Endangered Species

The NYSDEC Environmental Resource Mapper was reviewed for potential records of state or federally listed threatened or endangered species. The site is not within an area designated as a potential natural community for endangered species and no further review is warranted. The database results are included within Appendix C.

### Maps & Figures

Additional Maps indicating the site are included within Appendix D such as:

General Location Map

USGS Soils Map

### Best Management Practices

#### Objectives

The primary objective of the Stormwater Pollution Prevention Plan is protecting adjacent areas from erosion and sediment transport and ensuring the quality of discharge water is acceptable. This is done by minimizing disturbed areas, protecting natural features and soil, phasing construction, stabilizing soils, and protecting storm inlets.

#### Phasing

Construction of the subject site is intended to be conducted in multiple phases. Within the phases, attention should be paid to the required sequencing to ensure minimal sediment transport.

## Good Housekeeping

The owner/operator shall implement the following for the duration of construction:

- All stored materials shall be in a neat, orderly manner and under cover.
- Products shall be kept in original containers with a legible original manufacturer's label.
- Substances shall not be mixed with one another unless recommended by the manufacturer.
- Original labels and material safety data sheets (MSDS) shall be procured and used for each material.
- Whenever possible, the entire product shall be used up before disposing of a container.
- If surplus product must be disposed of, manufacturers or local/state/federal recommended methods for proper disposal shall be followed.
- Manufacturer's recommendations for proper use and disposal shall be followed.
- The job site superintendent shall be responsible for daily inspections to ensure proper use and disposal of materials.

## Spill Prevention Controls

The following spill prevention controls shall be implemented for the duration of construction:

- The job site superintendent shall be the spill prevention and cleanup coordinator. He/she shall designate the individuals who will receive spill prevention and cleanup training. These individuals shall each become responsible for a phase of prevention and cleanup. The names of these personnel shall be posted in the material storage area and in the office trailer onsite.
- Manufacturer's recommended methods for spill cleanup shall be clearly posted and site personnel shall be trained regarding these procedures as well as the location of the information and cleanup supplies.
- Materials and equipment necessary for spill cleanup shall be kept in the material storage area onsite in spill control and containment kit (containing, for example, absorbent such as kitty litter or sawdust, acid neutralizing powder, brooms, dust pans, mops, rags, gloves, goggles, plastic and metal trash containers, etc.).
- All spills shall be cleaned up immediately after discovery.
- The spill area shall be kept well ventilated, and personnel shall wear appropriate protective clothing to prevent injury from contact with the hazardous substances.
- Spills of toxic or hazardous materials shall be reported to the appropriate federal, state, and/or local government agency, regardless of the size of the spill. Spills of amounts that exceed Reportable Quantities of certain substances specifically mentioned in federal regulations (40 CFR 302 list and oil) shall be immediately reported to the EPA National Response Center, telephone 1-800-424-8802.

## Temporary Erosion & Sediment Controls

Temporary stormwater control measures shall be installed prior to active construction within each tributary area. Such temporary controls include but are not limited to:

- Silt fencing.
- Stabilized construction entrances.
- Once no longer active, disturbed areas shall be mulched to prevent sediment transport. Areas that are at or near finish grade shall be finally stabilized.

- Inlet protection devices shall be installed around all storm basins within active disturbance areas or areas not yet finally stabilized.
- Dust shall be controlled with water on site and adjacent roadways.
- Designate a protected area to stockpile topsoil or other material stripped during excavation. Stockpiles of soil materials shall be stabilized with geotextile or seeding and be surrounded by silt fencing or berming.
- No area shall be left unstabilized more than 14 days after completion of construction activities within that area.
- Erosion control devices should be cleaned and repaired as necessary.
- Other temporary erosion and sediment control devices including catch basin sediment traps as necessary.
- Litter and construction debris shall be collected daily by the contractor, and properly disposed of. Any refuse storage onsite shall be only in designated areas where runoff will not directly discharge through.

### Winter Shutdown

The site may be considered within 'winter shutdown' if the following conditions are met. During winter shutdown, the site inspection frequency may be reduced to once per 30-days. All disturbed areas shall be temporarily stabilized, and sediment basins shall be cleaned of silt and debris. During shutdown, access road shall be kept clear of snow and snow shall not be stockpiled in a location which inhibits runoff to sediment basin areas.

### Final Stabilization

Prior to the site being operational the following measures shall be implemented:

- All disturbed areas other than structures or pavement shall receive final seeding and vegetative growth.
- Inlets and structures shall be cleaned of silt for proper sump.
- Stormwater practices shall be finally shaped in accordance with the sizing details and shall be vegetated accordingly.
- Maintenance of ponds, swales and vegetative areas shall continue into operation of the site.

### Ownership & Maintenance

The owner/operator shall adhere to the Ownership and Maintenance Manual included within Appendix E.

### Inspections & Recordkeeping

#### Inspection Requirements

The owner/operation shall perform routine inspections and either correct or direct the contractor to correct deficiencies as they arise in a timely manner. The contractor shall familiarize themselves with this document and its required components prior to commencing work. Each day that the contractor is performing work on-site there shall be a 'trained individual' who is responsible for implementation of the SWPPP components.

The owner shall have a qualified inspector conduct a site inspection at least once per seven calendar days while disturbance activities are on-going. The inspector shall at a minimum, inspect erosion & sediment

control practices and pollution prevention measures to ensure integrity and effectiveness, all post-construction stormwater management practices under construction to ensure that they are constructed in conformance with the SWPPP, all areas of disturbance that have not achieved final stabilization, all points of discharge to natural surface waterbodies located within, or immediately adjacent to, the property boundaries of the construction site, and all points of discharge from the construction site.

The qualified inspector shall prepare an inspection report in accordance with the General Permit and distribute to the owner and appropriate contractor within 24 hours.

### Certifications

The SWPPP preparer, owner and contractor shall sign the applicable certification forms included within Appendix F.

### Documents Required On-Site

The owner or operator shall maintain a copy of the current General Permit, NOI, NOI Acknowledgment Letter, SWPPP, MS4 SWPPP Acceptance form, inspection reports, and all documentation necessary to demonstrate eligibility with this permit at the construction site until all disturbed areas have achieved final stabilization and the NOT has been submitted to the Department. The documents must be maintained in a secure location, such as a job trailer, on-site construction office, or mailbox with lock. The secure location must be accessible during normal business hours to an individual performing a compliance inspection.

### Drainage Analysis

#### Existing Runoff Condition

The existing site drainage characteristics including the existing structure and gravel were analyzed to determine baseline peak flow rates for the project. Stormwater runoff from the site was analyzed utilizing software applying the TR-55 hydrologic analysis method. The channel protection volume was determined utilizing the peak discharge from the TR-55 method and the Hydrologic Analysis tolls in Appendix B of the New York State Stormwater Management Design Manual. A summary of these peak flow rates is included below as well as the full drainage map & analysis within Appendix G.

Channel Protection Volume (acre-feet)	
	1-Year (Cpv)
Analysis Point A	0.00

Peak Flow Rates (CFS)		
	10-Year Storm (Qp)	100-Year Storm (Qf)
Analysis Point A	0.01	0.44

#### Proposed Development Condition

The proposed site drainage characteristics were analyzed in relation to the existing baseline to determine required storage volumes for the site. Changes in impervious cover, sub-catchment area and times of concentration were all considered in conducting the analysis. A summary of these peak flow rates is included below as well as the full drainage map & analysis within Appendix H.



Channel Protection Volume (acre-feet)		
	EX 1-Year Storm (Cpv)	PR 1-Year Storm (Cpv)
Analysis Point A	0.00	0.00

Peak Flow Rates (CFS)				
	EX 10-Year Storm (Qp)	PR 10-Year Storm (Qp)	EX 100-Year Storm (Qf)	PR 100-Year Storm (Qf)
Analysis Point A	0.01	0.01	0.44	0.42

## Water Quality & Quantity Controls

### Selecting Post-Construction Practices

Post-construction stormwater management practices were carefully selected considering the matrices provided by the NYS DEC Stormwater Management Design Manual. Screening factors included 1. Land Use 2. Physical Feasibility 3. Watershed/Regional Factors 4. Stormwater Management Capability 5. Community & Environmental Factors.

Part of the consideration in selecting stormwater practices was the runoff reduction capacity of the practice. In accordance with the NYSDEC General Permit and Stormwater Design Manual each site must meet the minimum runoff reduction requirement through a combination of Green Infrastructure Practices and SMP's with runoff reduction capacity.

### Water Quality

Practices selected for treatment of water quality include:

#### Dry Swale

All water quality practices have been designed to treat the calculated water quality volume as well as safely convey the 10-year storm event. Worksheets showing sizing criteria and calculations are included within Appendix H.

### Water Quantity

Stormwater controls for water quantity include:

#### Dry Swale

Water quantity practices have been designed to attenuate flows from both the Overbank Flood (10-year) and the Extreme Flood (100-year) storm events. The proposed stormwater detention areas do not meet the requirements for consideration as a "dam" as prescribed by NYSDEC. It can be assumed that in the unlikely event for a failure or misoperation losses would be limited to the owner's property. Pond storage elevation and sizing information is included in the post development drainage calculations within Appendix G.

## Conclusion

The subject project includes expansion of the existing structure and construction of driveways, parking, and recreational facilities. This activity is listed within Appendix B Table 2 of the NYSDEC General Permit 0-20-001 for stormwater discharges from construction activities.

Any future modifications made to the plan to accommodate additional structures is not expected to require any impoundments, catch basins or drainage off-site, including the use of culverts on public highways.

Appendix A

Notice of Intent

# NOI for coverage under Stormwater General Permit for Construction Activity

version 1.30

(Submission #: HP9-9AR6-81QPM, version 1)

## Details

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**Originally Started By** CHRISTOPHER LONGO

**Submission ID** HP9-9AR6-81QPM

**Submission Reason** New

**Status** Draft

## Form Input

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### Owner/Operator Information

**Owner/Operator Name (Company/Private Owner/Municipality/Agency/Institution, etc.)**

Caroga Arts Collective, Inc.

**Owner/Operator Contact Person Last Name (NOT CONSULTANT)**

Ruby

**Owner/Operator Contact Person First Name**

Rick

**Owner/Operator Mailing Address**

P.O. Box 1048

**City**

Caroga Lake

**State**

NY

**Zip**

12032

**Phone**

518-844-2679

**Email**

rruby@randq.com

**Federal Tax ID**

NONE PROVIDED

**Project Location****Project/Site Name**

Caroga Lake Arts Collective

**Street Address (Not P.O. Box)**

1989 State Hwy 10

**Side of Street**

East

**City/Town/Village (THAT ISSUES BUILDING PERMIT)**

Town of Caroga

**State**

NY

**Zip**

12032

**DEC Region**

5

**County**

FULTON

**Name of Nearest Cross Street**

County Route 112

**Distance to Nearest Cross Street (Feet)**

285

**Project In Relation to Cross Street**

North

**Tax Map Numbers Section-Block-Parcel**

68.17-2-1, 68.17-2-2

**Tax Map Numbers**

NONE PROVIDED

**1. Coordinates**

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Provide the Geographic Coordinates for the project site. The two methods are:

- Navigate to the project location on the map (below) and click to place a marker and obtain the XY coordinates.
- The "Find Me" button will provide the lat/long for the person filling out this form. Then pan the map to the correct location and click the map to place a marker and obtain the XY coordinates.

**Navigate to your location and click on the map to get the X,Y coordinates**

43.1467857,-74.48674009999999

**Project Details****2. What is the nature of this project?**

Redevelopment with increase in impervious area

**3. Select the predominant land use for both pre and post development conditions.****Pre-Development Existing Landuse**

Forest

**Post-Development Future Land Use**

Commercial

**3a. If Single Family Subdivision was selected in question 3, enter the number of subdivision lots.**

NONE PROVIDED

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4. In accordance with the larger common plan of development or sale, enter the total project site acreage, the acreage to be disturbed and the future impervious area (acreage)within the disturbed area.

\*\*\* ROUND TO THE NEAREST TENTH OF AN ACRE. \*\*\*

**Total Site Area (acres)**

10.67

**Total Area to be Disturbed (acres)**

1.4

**Existing Impervious Area to be Disturbed (acres)**

.13

**Future Impervious Area Within Disturbed Area (acres)**

.27

**5. Do you plan to disturb more than 5 acres of soil at any one time?**

No

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**6. Indicate the percentage (%) of each Hydrologic Soil Group(HSG) at the site.****A (%)**

0

**B (%)**

5

**C (%)**

95

**D (%)**

0

**7. Is this a phased project?**

Yes

**8. Enter the planned start and end dates of the disturbance activities.****Start Date**

9/1/2021

**End Date**

10/31/2022

**9. Identify the nearest surface waterbody(ies) to which construction site runoff will discharge.**

Wetland

**9a. Type of waterbody identified in question 9?**

Wetland/State Jurisdiction On Site (Answer 9b)

**Other Waterbody Type Off Site Description**

NONE PROVIDED

**9b. If "wetland" was selected in 9A, how was the wetland identified?**

NONE PROVIDED

**10. Has the surface waterbody(ies) in question 9 been identified as a 303(d) segment in Appendix E of GP-0-20-001?**

No

**11. Is this project located in one of the Watersheds identified in Appendix C of GP-0-20-001?**

No

**12. Is the project located in one of the watershed areas associated with AA and AA-S classified waters?**

No

**If No, skip question 13.**

**13. Does this construction activity disturb land with no existing impervious cover and where the Soil Slope Phase is identified as an E or F on the USDA Soil Survey?**

No

**If Yes, what is the acreage to be disturbed?**

NONE PROVIDED

**14. Will the project disturb soils within a State regulated wetland or the protected 100 foot adjacent area?**

No

**15. Does the site runoff enter a separate storm sewer system (including roadside drains, swales, ditches, culverts, etc)?**

No

**16. What is the name of the municipality/entity that owns the separate storm sewer system?**

N/A

**17. Does any runoff from the site enter a sewer classified as a Combined Sewer?**

No

**18. Will future use of this site be an agricultural property as defined by the NYS Agriculture and Markets Law?**

No

**19. Is this property owned by a state authority, state agency, federal government or local government?**

No

**20. Is this a remediation project being done under a Department approved work plan? (i.e. CERCLA, RCRA, Voluntary Cleanup Agreement, etc.)**

No

## **Required SWPPP Components**



**21. Has the required Erosion and Sediment Control component of the SWPPP been developed in conformance with the current NYS Standards and Specifications for Erosion and Sediment Control (aka Blue Book)?**

Yes

**22. Does this construction activity require the development of a SWPPP that includes the post-construction stormwater management practice component (i.e. Runoff Reduction, Water Quality and Quantity Control practices/techniques)?**

Yes

**If you answered No in question 22, skip question 23 and the Post-construction Criteria and Post-construction SMP Identification sections.**

**23. Has the post-construction stormwater management practice component of the SWPPP been developed in conformance with the current NYS Stormwater Management Design Manual?**

Yes

**24. The Stormwater Pollution Prevention Plan (SWPPP) was prepared by:**  
Professional Engineer (P.E.)

**SWPPP Preparer**

Empire Engineering, PLLC

**Contact Name (Last, Space, First)**

Longo, Christopher, PE

**Mailing Address**

1900 Duanesburg Rd.

**City**

Duanesburg

**State**

NY

**Zip**

12056

**Phone**

518-280-1371

**Email**

CLongo@EmpireEng.net

**Download SWPPP Preparer Certification Form**

Please take the following steps to prepare and upload your preparer certification form:

- 1) Click on the link below to download a blank certification form
- 2) The certified SWPPP preparer should sign this form

3) Scan the signed form

4) Upload the scanned document

[Download SWPPP Preparer Certification Form](#)

**Please upload the SWPPP Preparer Certification**

NONE PROVIDED

**Comment**

NONE PROVIDED

## **Erosion & Sediment Control Criteria**

**25. Has a construction sequence schedule for the planned management practices been prepared?**

Yes

**26. Select all of the erosion and sediment control practices that will be employed on the project site:**

### **Temporary Structural**

Construction Road Stabilization

Dust Control

Silt Fence

Stabilized Construction Entrance

Temporary Swale

### **Biotechnical**

None

### **Vegetative Measures**

Seeding

Temporary Swale

Mulching

### **Permanent Structural**

Rock Outlet Protection

### **Other**

NONE PROVIDED

## **Post-Construction Criteria**

**\* IMPORTANT: Completion of Questions 27-39 is not required if response to Question 22 is No.**

**27. Identify all site planning practices that were used to prepare the final site plan/layout for the project.**

Preservation of Undisturbed Area  
 Preservation of Buffers  
 Reduction of Clearing and Grading  
 Locating Development in Less Sensitive Areas  
 Roadway Reduction  
 Building Footprint Reduction

**27a. Indicate which of the following soil restoration criteria was used to address the requirements in Section 5.1.6("Soil Restoration") of the Design Manual (2010 version).**

All disturbed areas will be restored in accordance with the Soil Restoration requirements in Table 5.3 of the Design Manual (see page 5-22).

**28. Provide the total Water Quality Volume (WQv) required for this project (based on final site plan/layout). (Acre-feet)**

.02

**29. Post-construction SMP Identification**

Use the Post-construction SMP Identification section to identify the RR techniques (Area Reduction), RR techniques(Volume Reduction) and Standard SMPs with RRv Capacity that were used to reduce the Total WQv Required (#28).

Identify the SMPs to be used by providing the total impervious area that contributes runoff to each technique/practice selected. For the Area Reduction Techniques, provide the total contributing area (includes pervious area) and, if applicable, the total impervious area that contributes runoff to the technique/practice.

Note: Redevelopment projects shall use the Post-Construction SMP Identification section to identify the SMPs used to treat and/or reduce the WQv required. If runoff reduction techniques will not be used to reduce the required WQv, skip to question 33a after identifying the SMPs.

**30. Indicate the Total RRv provided by the RR techniques (Area/Volume Reduction) and Standard SMPs with RRv capacity identified in question 29. (acre-feet)**

.005

**31. Is the Total RRv provided (#30) greater than or equal to the total WQv required (#28)?**

No

**If Yes, go to question 36. If No, go to question 32.**

**32. Provide the Minimum RRv required based on HSG. [Minimum RRv Required = (P) (0.95) (Ai) / 12, Ai=(s) (Aic)] (acre-feet)**

.004

**32a. Is the Total RRv provided (#30) greater than or equal to the Minimum RRv Required (#32)?**

Yes

**If Yes, go to question 33.**

Note: Use the space provided in question #39 to summarize the specific site limitations and justification for not reducing 100% of WQv required (#28). A detailed evaluation of the specific site limitations and justification for not reducing 100% of the WQv required (#28) must also be included in the SWPPP.

If No, sizing criteria has not been met; therefore, NOI can not be processed. SWPPP preparer must modify design to meet sizing criteria.

**33. SMPs**

Use the Post-construction SMP Identification section to identify the Standard SMPs and, if applicable, the Alternative SMPs to be used to treat the remaining total WQv (=Total WQv Required in #28 - Total RRv Provided in #30).

Also, provide the total impervious area that contributes runoff to each practice selected.

NOTE: Use the Post-construction SMP Identification section to identify the SMPs used on Redevelopment projects.

**33a. Indicate the Total WQv provided (i.e. WQv treated) by the SMPs identified in question #33 and Standard SMPs with RRv Capacity identified in question #29. (acre-feet)**

.015

Note: For the standard SMPs with RRv capacity, the WQv provided by each practice = the WQv calculated using the contributing drainage area to the practice - provided by the practice. (See Table 3.5 in Design Manual)

**34. Provide the sum of the Total RRv provided (#30) and the WQv provided (#33a).**

.02

**35. Is the sum of the RRv provided (#30) and the WQv provided (#33a) greater than or equal to the total WQv required (#28)?**

Yes

If Yes, go to question 36.

If No, sizing criteria has not been met; therefore, NOI can not be processed. SWPPP preparer must modify design to meet sizing criteria.

**36. Provide the total Channel Protection Storage Volume (CPv required and provided or select waiver (#36a), if applicable.**

**CPv Required (acre-feet)**

NONE PROVIDED

**CPv Provided (acre-feet)**

NONE PROVIDED

**36a. The need to provide channel protection has been waived because:**

Reduction of the total CPv is achieved on site through runoff reduction techniques or infiltration systems.

**37. Provide the Overbank Flood (Qp) and Extreme Flood (Qf) control criteria or select waiver (#37a), if applicable.****Overbank Flood Control Criteria (Qp)****Pre-Development (CFS)**

.01

**Post-Development (CFS)**

.01

**Total Extreme Flood Control Criteria (Qf)****Pre-Development (CFS)**

.44

**Post-Development (CFS)**

.42

**37a. The need to meet the Qp and Qf criteria has been waived because:**

NONE PROVIDED

**38. Has a long term Operation and Maintenance Plan for the post-construction stormwater management practice(s) been developed?**

Yes

**If Yes, Identify the entity responsible for the long term Operation and Maintenance**

Private Owner

**39. Use this space to summarize the specific site limitations and justification for not reducing 100% of WQv required (#28). (See question #32a) This space can also be used for other pertinent project information.**

Redevelopment of an existing paved site and existing building.

**Post-Construction SMP Identification****Runoff Reduction (RR) Techniques, Standard Stormwater Management Practices (SMPs) and Alternative SMPs**

Identify the Post-construction SMPs to be used by providing the total impervious area that contributes runoff to each technique/practice selected. For the Area Reduction Techniques, provide the total contributing area (includes pervious area) and, if applicable, the total impervious area that contributes runoff to the technique/practice.

**RR Techniques (Area Reduction)**

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Round to the nearest tenth

**Total Contributing Acres for Conservation of Natural Area (RR-1)**

NONE PROVIDED

**Total Contributing Impervious Acres for Conservation of Natural Area (RR-1)**

NONE PROVIDED

**Total Contributing Acres for Sheetflow to Riparian Buffers/Filter Strips (RR-2)**

NONE PROVIDED

**Total Contributing Impervious Acres for Sheetflow to Riparian Buffers/Filter Strips (RR-2)**

NONE PROVIDED

**Total Contributing Acres for Tree Planting/Tree Pit (RR-3)**

NONE PROVIDED

**Total Contributing Impervious Acres for Tree Planting/Tree Pit (RR-3)**

NONE PROVIDED

**Total Contributing Acres for Disconnection of Rooftop Runoff (RR-4)**

NONE PROVIDED

**RR Techniques (Volume Reduction)**

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**Total Contributing Impervious Acres for Disconnection of Rooftop Runoff (RR-4)**

NONE PROVIDED

**Total Contributing Impervious Acres for Vegetated Swale (RR-5)**

NONE PROVIDED

**Total Contributing Impervious Acres for Rain Garden (RR-6)**

NONE PROVIDED

**Total Contributing Impervious Acres for Stormwater Planter (RR-7)**

NONE PROVIDED

**Total Contributing Impervious Acres for Rain Barrel/Cistern (RR-8)**

NONE PROVIDED

**Total Contributing Impervious Acres for Porous Pavement (RR-9)**

NONE PROVIDED

**Total Contributing Impervious Acres for Green Roof (RR-10)**

NONE PROVIDED

**Standard SMPs with RRV Capacity**

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**Total Contributing Impervious Acres for Infiltration Trench (I-1)**

NONE PROVIDED

**Total Contributing Impervious Acres for Infiltration Basin (I-2)**

NONE PROVIDED

**Total Contributing Impervious Acres for Dry Well (I-3)**

NONE PROVIDED

**Total Contributing Impervious Acres for Underground Infiltration System (I-4)**

NONE PROVIDED

**Total Contributing Impervious Acres for Bioretention (F-5)**

NONE PROVIDED

**Total Contributing Impervious Acres for Dry Swale (O-1)**

.27

**Standard SMPs**

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**Total Contributing Impervious Acres for Micropool Extended Detention (P-1)**

NONE PROVIDED

**Total Contributing Impervious Acres for Wet Pond (P-2)**

NONE PROVIDED

**Total Contributing Impervious Acres for Wet Extended Detention (P-3)**

NONE PROVIDED

**Total Contributing Impervious Acres for Multiple Pond System (P-4)**

NONE PROVIDED

**Total Contributing Impervious Acres for Pocket Pond (P-5)**

NONE PROVIDED

**Total Contributing Impervious Acres for Surface Sand Filter (F-1)**

NONE PROVIDED

**Total Contributing Impervious Acres for Underground Sand Filter (F-2)**

NONE PROVIDED

**Total Contributing Impervious Acres for Perimeter Sand Filter (F-3)**

NONE PROVIDED

**Total Contributing Impervious Acres for Organic Filter (F-4)**

NONE PROVIDED

**Total Contributing Impervious Acres for Shallow Wetland (W-1)**

NONE PROVIDED

**Total Contributing Impervious Acres for Extended Detention Wetland (W-2)**

NONE PROVIDED

**Total Contributing Impervious Acres for Pond/Wetland System (W-3)**

NONE PROVIDED

**Total Contributing Impervious Acres for Pocket Wetland (W-4)**

NONE PROVIDED

**Total Contributing Impervious Acres for Wet Swale (O-2)**

NONE PROVIDED

**Alternative SMPs (DO NOT INCLUDE PRACTICES BEING USED FOR PRETREATMENT ONLY)**

---

**Total Contributing Impervious Area for Hydrodynamic**

NONE PROVIDED

**Total Contributing Impervious Area for Wet Vault**

NONE PROVIDED

**Total Contributing Impervious Area for Media Filter**

NONE PROVIDED

**"Other" Alternative SMP?**

NONE PROVIDED

**Total Contributing Impervious Area for "Other"**

NONE PROVIDED

**Provide the name and manufacturer of the alternative SMPs (i.e. proprietary practice(s)) being used for WQv treatment.**

**Note: Redevelopment projects which do not use RR techniques, shall use questions 28, 29, 33 and 33a to provide SMPs used, total WQv required and total WQv provided for the project.**

**Manufacturer of Alternative SMP**

NONE PROVIDED

**Name of Alternative SMP**

NONE PROVIDED

**Other Permits**



**40. Identify other DEC permits, existing and new, that are required for this project/facility.**

None

**If SPDES Multi-Sector GP, then give permit ID**

NONE PROVIDED

**If Other, then identify**

NONE PROVIDED

**41. Does this project require a US Army Corps of Engineers Wetland Permit?**

Yes

**If "Yes," then indicate Size of Impact, in acres, to the nearest tenth**

.1

**42. If this NOI is being submitted for the purpose of continuing or transferring coverage under a general permit for stormwater runoff from construction activities, please indicate the former SPDES number assigned.**

NONE PROVIDED

## **MS4 SWPPP Acceptance**

**43. Is this project subject to the requirements of a regulated, traditional land use control MS4?**

No

**If No, skip question 44**

**44. Has the "MS4 SWPPP Acceptance" form been signed by the principal executive officer or ranking elected official and submitted along with this NOI?**

NONE PROVIDED

### **MS4 SWPPP Acceptance Form Download**

Download form from the link below. Complete, sign, and upload.

[MS4 SWPPP Acceptance Form](#)

### **MS4 Acceptance Form Upload**

NONE PROVIDED

**Comment**

NONE PROVIDED

## **Owner/Operator Certification**

### **Owner/Operator Certification Form Download**

Download the certification form by clicking the link below. Complete, sign, scan, and upload the form.

[Owner/Operator Certification Form \(PDF, 45KB\)](#)

**Upload Owner/Operator Certification Form**

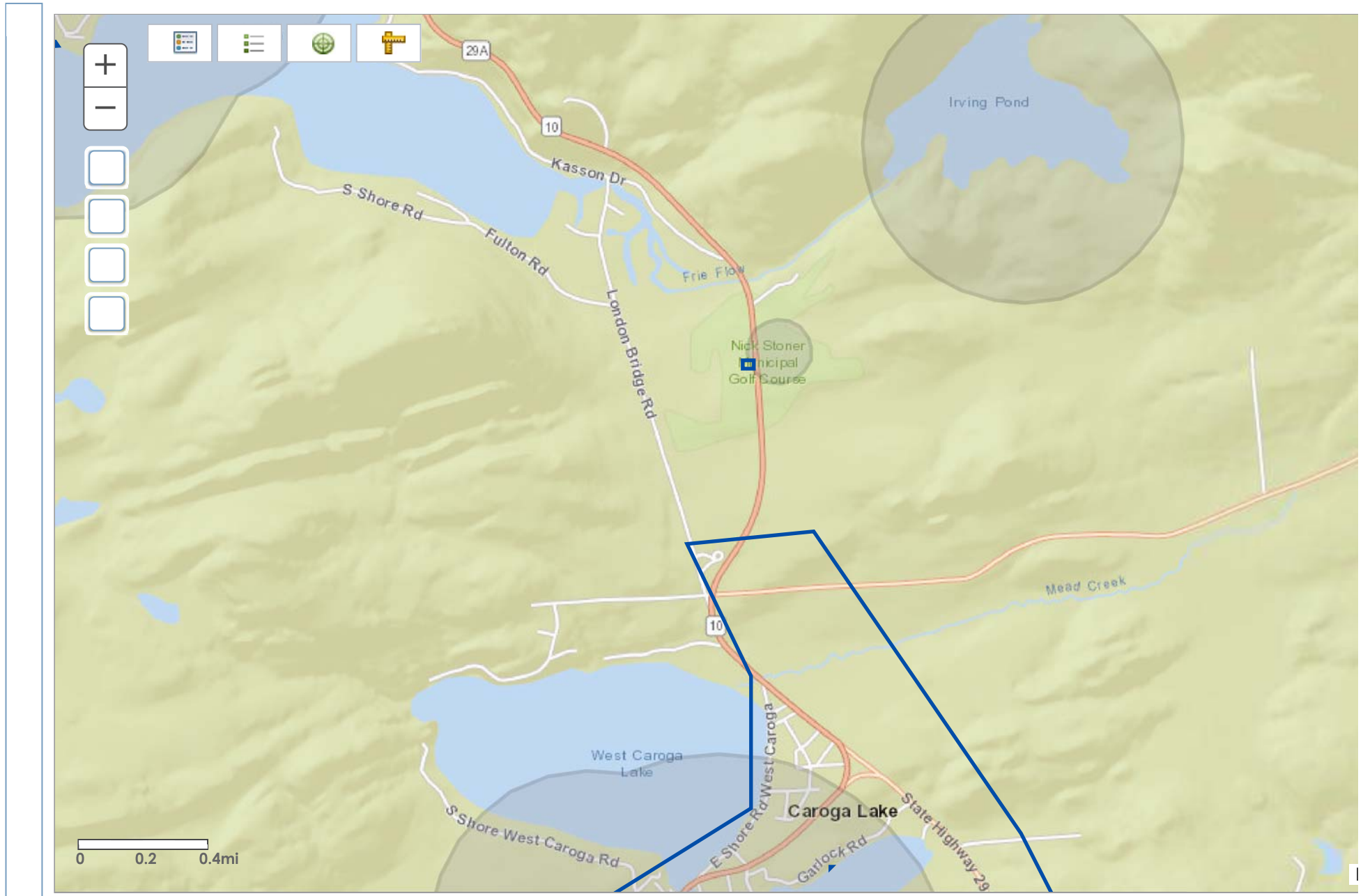
NONE PROVIDED

**Comment**

NONE PROVIDED

## Appendix B

### OPRHP Correspondence



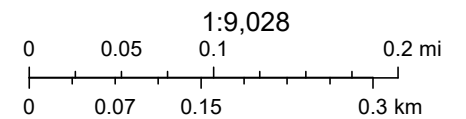
## Appendix C

### T&E Correspondence

# Environmental Resource Mapper



June 7, 2021



Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri

NYS Department of Environmental Conservation  
Not a legal document

## Appendix D

### Maps & Figures



# Custom Soil Resource Report Soil Map






# Custom Soil Resource Report


## MAP LEGEND

### Area of Interest (AOI)

 Area of Interest (AOI)


### Soils


 Soil Map Unit Polygons


 Soil Map Unit Lines


 Soil Map Unit Points

### Special Point Features

 Blowout

 Borrow Pit


 Clay Spot


 Closed Depression

 Gravel Pit

 Gravelly Spot

 Landfill

 Lava Flow

 Marsh or swamp

 Mine or Quarry

 Miscellaneous Water

 Perennial Water

 Rock Outcrop

 Saline Spot

 Sandy Spot

 Severely Eroded Spot


 Sinkhole


 Slide or Slip

 Sodic Spot


 Spoil Area

 Stony Spot


 Very Stony Spot

 Wet Spot

 Other

 Special Line Features

### Water Features

 Streams and Canals


### Transportation

 Rails


 Interstate Highways

 US Routes

 Major Roads

 Local Roads

### Background

 Aerial Photography

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
Web Soil Survey URL:  
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Fulton County, New York  
Survey Area Data: Version 20, Jun 11, 2020

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 8, 2019—Aug 2, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
1170C	Henniker fine sandy loam, 8 to 15 percent slopes	12.1	86.9%
1171B	Metacomet fine sandy loam, 3 to 8 percent slopes	1.1	7.6%
1920E	Monadnock fine sandy loam, 15 to 35 percent slopes, very bouldery	0.8	5.4%
<b>Totals for Area of Interest</b>		<b>14.0</b>	<b>100.0%</b>

## Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or

landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

## Fulton County, New York

### 1170C—Henniker fine sandy loam, 8 to 15 percent slopes

#### Map Unit Setting

*National map unit symbol:* bqnp

*Elevation:* 1,000 to 2,820 feet

*Mean annual precipitation:* 35 to 50 inches

*Mean annual air temperature:* 37 to 43 degrees F

*Frost-free period:* 90 to 130 days

*Farmland classification:* Farmland of statewide importance

#### Map Unit Composition

*Henniker and similar soils:* 80 percent

*Minor components:* 20 percent

*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### Description of Henniker

##### Setting

*Landform:* Till plains

*Landform position (two-dimensional):* Shoulder

*Landform position (three-dimensional):* Side slope

*Down-slope shape:* Convex

*Across-slope shape:* Convex

*Parent material:* Friable loamy till underlain by firm sandy lodgment till derived from igneous and metamorphic rock

##### Typical profile

*Oe - 0 to 2 inches:* moderately decomposed plant material

*Ap - 2 to 8 inches:* fine sandy loam

*Bw1 - 8 to 20 inches:* gravelly fine sandy loam

*Bw2 - 20 to 31 inches:* gravelly fine sandy loam

*Cd1 - 31 to 52 inches:* gravelly loamy fine sand

*Cd2 - 52 to 72 inches:* gravelly fine sandy loam

##### Properties and qualities

*Slope:* 8 to 15 percent

*Surface area covered with cobbles, stones or boulders:* 0.0 percent

*Depth to restrictive feature:* 20 to 40 inches to densic material

*Drainage class:* Well drained

*Runoff class:* Medium

*Capacity of the most limiting layer to transmit water (Ksat):* Moderately low to moderately high (0.06 to 0.57 in/hr)

*Depth to water table:* About 28 to 40 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Available water capacity:* Low (about 4.5 inches)

##### Interpretive groups

*Land capability classification (irrigated):* None specified

*Land capability classification (nonirrigated):* 3e

*Hydrologic Soil Group:* C

*Hydric soil rating:* No

**Minor Components**

**Becket**

*Percent of map unit: 8 percent*  
*Hydric soil rating: No*

**Metacomet**

*Percent of map unit: 5 percent*  
*Hydric soil rating: No*

**Skerry**

*Percent of map unit: 3 percent*  
*Hydric soil rating: No*

**Monadnock**

*Percent of map unit: 2 percent*  
*Hydric soil rating: No*

**Unnamed, stony**

*Percent of map unit: 1 percent*  
*Hydric soil rating: No*

**Pillsbury**

*Percent of map unit: 1 percent*  
*Hydric soil rating: No*

**1171B—Metacomet fine sandy loam, 3 to 8 percent slopes**

**Map Unit Setting**

*National map unit symbol: bqnm*  
*Elevation: 1,000 to 2,820 feet*  
*Mean annual precipitation: 35 to 50 inches*  
*Mean annual air temperature: 37 to 43 degrees F*  
*Frost-free period: 90 to 130 days*  
*Farmland classification: All areas are prime farmland*

**Map Unit Composition**

*Metacomet and similar soils: 80 percent*  
*Minor components: 20 percent*  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

**Description of Metacomet**

**Setting**

*Landform: Till plains*  
*Landform position (two-dimensional): Summit*  
*Landform position (three-dimensional): Head slope*  
*Down-slope shape: Concave*  
*Across-slope shape: Convex*  
*Parent material: Friable loamy till underlain by firm sandy lodgment till derived from igneous and metamorphic rock*

## Custom Soil Resource Report

### Typical profile

*Oe - 0 to 2 inches:* moderately decomposed plant material  
*Ap - 2 to 8 inches:* fine sandy loam  
*Bw - 8 to 20 inches:* fine sandy loam  
*BC - 20 to 27 inches:* fine sandy loam  
*C - 27 to 31 inches:* cobbly loamy sand  
*Cd - 31 to 45 inches:* gravelly loamy sand  
*C' - 45 to 72 inches:* cobbly loamy sand

### Properties and qualities

*Slope:* 3 to 8 percent  
*Surface area covered with cobbles, stones or boulders:* 0.0 percent  
*Depth to restrictive feature:* 20 to 38 inches to densic material  
*Drainage class:* Moderately well drained  
*Runoff class:* Medium  
*Capacity of the most limiting layer to transmit water (Ksat):* Moderately low to moderately high (0.06 to 0.60 in/hr)  
*Depth to water table:* About 18 to 30 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Available water capacity:* Low (about 4.0 inches)

### Interpretive groups

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 2w  
*Hydrologic Soil Group:* C/D  
*Hydric soil rating:* No

### Minor Components

#### Pillsbury

*Percent of map unit:* 8 percent  
*Landform:* Ground moraines  
*Hydric soil rating:* No

#### Skerry

*Percent of map unit:* 5 percent  
*Hydric soil rating:* No

#### Henniker

*Percent of map unit:* 3 percent  
*Hydric soil rating:* No

#### Adirondack

*Percent of map unit:* 2 percent  
*Hydric soil rating:* No

#### Unnamed

*Percent of map unit:* 2 percent  
*Hydric soil rating:* No

## **1920E—Monadnock fine sandy loam, 15 to 35 percent slopes, very bouldery**

### **Map Unit Setting**

*National map unit symbol:* bqnv

*Elevation:* 1,000 to 2,820 feet

*Mean annual precipitation:* 35 to 50 inches

*Mean annual air temperature:* 37 to 43 degrees F

*Frost-free period:* 90 to 130 days

*Farmland classification:* Not prime farmland

### **Map Unit Composition**

*Monadnock, very bouldery, and similar soils:* 80 percent

*Minor components:* 20 percent

*Estimates are based on observations, descriptions, and transects of the mapunit.*

### **Description of Monadnock, Very Bouldery**

#### **Setting**

*Landform:* Moraines, valley sides

*Landform position (two-dimensional):* Backslope

*Landform position (three-dimensional):* Side slope

*Down-slope shape:* Convex

*Across-slope shape:* Convex

*Parent material:* A loamy mantle underlain by acid sandy and gravelly till derived mainly from crystalline rock

#### **Typical profile**

*Oi - 0 to 1 inches:* slightly decomposed plant material

*A - 1 to 2 inches:* fine sandy loam

*E - 2 to 7 inches:* sandy loam

*Bs - 7 to 14 inches:* fine sandy loam

*BC - 14 to 27 inches:* gravelly fine sandy loam

*2C1 - 27 to 41 inches:* very gravelly loamy sand

*2C2 - 41 to 72 inches:* gravelly loamy sand

#### **Properties and qualities**

*Slope:* 15 to 35 percent

*Surface area covered with cobbles, stones or boulders:* 1.6 percent

*Depth to restrictive feature:* More than 80 inches

*Drainage class:* Well drained

*Runoff class:* High

*Capacity of the most limiting layer to transmit water (Ksat):* Moderately high to high  
(0.20 to 1.98 in/hr)

*Depth to water table:* More than 80 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Available water capacity:* Moderate (about 6.1 inches)

## Custom Soil Resource Report

### **Interpretive groups**

*Land capability classification (irrigated):* None specified

*Land capability classification (nonirrigated):* 7s

*Hydrologic Soil Group:* B

*Hydric soil rating:* No

### **Minor Components**

#### **Adams**

*Percent of map unit:* 9 percent

*Hydric soil rating:* No

#### **Becket**

*Percent of map unit:* 5 percent

*Hydric soil rating:* No

#### **Colton**

*Percent of map unit:* 5 percent

*Hydric soil rating:* No

#### **Skerry**

*Percent of map unit:* 1 percent

*Hydric soil rating:* No



# Extreme Precipitation Tables

## Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Smoothing	Yes
State	New York
Location	
Longitude	74.486 degrees West
Latitude	43.146 degrees North
Elevation	0 feet
Date/Time	Sun, 06 Jun 2021 21:38:34 -0400

## Extreme Precipitation Estimates

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.26	0.40	0.50	0.65	0.81	1.01	1yr	0.70	0.93	1.16	1.44	1.77	2.19	2.50	1yr	1.94	2.41	2.84	3.45	4.02	1yr
2yr	0.32	0.49	0.60	0.80	1.00	1.24	2yr	0.86	1.10	1.42	1.73	2.09	2.52	2.86	2yr	2.23	2.75	3.22	3.89	4.46	2yr
5yr	0.37	0.58	0.72	0.97	1.24	1.55	5yr	1.07	1.37	1.77	2.15	2.57	3.06	3.49	5yr	2.71	3.35	3.89	4.59	5.26	5yr
10yr	0.42	0.65	0.83	1.12	1.46	1.84	10yr	1.26	1.61	2.10	2.53	3.01	3.54	4.05	10yr	3.13	3.90	4.50	5.21	5.97	10yr
25yr	0.49	0.78	0.99	1.37	1.82	2.29	25yr	1.57	1.99	2.62	3.15	3.71	4.30	4.95	25yr	3.81	4.76	5.45	6.16	7.04	25yr
50yr	0.56	0.89	1.14	1.60	2.15	2.72	50yr	1.86	2.34	3.11	3.71	4.34	4.98	5.76	50yr	4.41	5.54	6.30	6.99	7.98	50yr
100yr	0.63	1.02	1.31	1.86	2.54	3.22	100yr	2.19	2.76	3.68	4.38	5.07	5.78	6.71	100yr	5.11	6.46	7.29	7.94	9.06	100yr
200yr	0.72	1.18	1.52	2.18	3.01	3.82	200yr	2.59	3.25	4.36	5.15	5.93	6.70	7.82	200yr	5.93	7.52	8.44	9.02	10.28	200yr
500yr	0.86	1.42	1.85	2.68	3.76	4.78	500yr	3.24	4.04	5.45	6.39	7.30	8.16	9.58	500yr	7.22	9.21	10.24	10.68	12.15	500yr

## Lower Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.22	0.35	0.42	0.57	0.70	0.87	1yr	0.60	0.85	0.97	1.22	1.58	2.01	2.32	1yr	1.78	2.24	2.67	3.22	3.75	1yr
2yr	0.30	0.46	0.56	0.76	0.94	1.09	2yr	0.81	1.07	1.21	1.58	1.97	2.47	2.78	2yr	2.19	2.68	3.16	3.80	4.38	2yr
5yr	0.34	0.52	0.65	0.89	1.14	1.29	5yr	0.98	1.26	1.46	1.86	2.31	2.90	3.27	5yr	2.57	3.14	3.67	4.38	4.99	5yr
10yr	0.38	0.58	0.72	1.01	1.30	1.45	10yr	1.12	1.42	1.67	2.09	2.59	3.29	3.69	10yr	2.92	3.55	4.12	4.86	5.50	10yr
25yr	0.44	0.66	0.82	1.18	1.55	1.70	25yr	1.34	1.67	1.96	2.40	3.01	3.89	4.31	25yr	3.45	4.14	4.78	5.56	6.25	25yr
50yr	0.48	0.73	0.91	1.31	1.76	1.92	50yr	1.52	1.88	2.23	2.69	3.37	4.40	4.84	50yr	3.90	4.65	5.34	6.15	6.88	50yr
100yr	0.54	0.81	1.01	1.47	2.01	2.17	100yr	1.73	2.12	2.53	3.01	3.77	5.02	5.43	100yr	4.44	5.23	6.00	6.81	7.58	100yr
200yr	0.60	0.90	1.14	1.65	2.30	2.45	200yr	1.98	2.40	2.87	3.38	4.23	5.70	6.10	200yr	5.05	5.87	6.69	7.56	8.34	200yr
500yr	0.69	1.03	1.33	1.93	2.75	2.89	500yr	2.37	2.82	3.40	3.95	4.90	6.78	7.09	500yr	6.00	6.82	7.74	8.64	9.45	500yr

## Upper Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.28	0.44	0.53	0.72	0.88	1.02	1yr	0.76	1.00	1.16	1.51	1.89	2.34	2.68	1yr	2.07	2.58	3.03	3.63	4.29	1yr
2yr	0.32	0.50	0.62	0.83	1.03	1.16	2yr	0.89	1.13	1.31	1.67	2.09	2.58	2.94	2yr	2.28	2.83	3.32	3.98	4.57	2yr
5yr	0.40	0.62	0.76	1.05	1.33	1.48	5yr	1.15	1.45	1.67	2.12	2.60	3.20	3.68	5yr	2.84	3.54	4.11	4.83	5.51	5yr
10yr	0.48	0.74	0.91	1.28	1.65	1.79	10yr	1.42	1.75	2.03	2.53	3.10	3.80	4.39	10yr	3.37	4.22	4.86	5.58	6.36	10yr
25yr	0.62	0.94	1.17	1.67	2.20	2.30	25yr	1.90	2.25	2.66	3.15	3.91	4.74	5.52	25yr	4.19	5.31	6.05	6.80	7.68	25yr
50yr	0.74	1.13	1.41	2.02	2.72	2.78	50yr	2.35	2.72	3.26	3.75	4.66	5.58	6.59	50yr	4.94	6.33	7.14	7.89	8.86	50yr
100yr	0.90	1.36	1.70	2.46	3.37	3.37	100yr	2.91	3.30	3.98	4.47	5.56	6.61	7.88	100yr	5.85	7.58	8.47	9.16	10.24	100yr
200yr	1.09	1.64	2.07	3.00	4.19	4.09	200yr	3.61	4.00	4.88	5.34	6.64	7.80	9.42	200yr	6.91	9.06	10.04	10.62	11.84	200yr
500yr	1.41	2.10	2.70	3.92	5.57	5.28	500yr	4.81	5.16	6.36	6.75	8.40	9.74	11.94	500yr	8.62	11.48	12.60	12.94	14.34	500yr

Appendix E

O&M Manual

# Operation & Maintenance Manual

For

Caroga Arts Collective, Inc.

Stormwater Management Facilities

At

1989 State Highway 10

Caroga, NY

## Site Information

The subject project is the commercial development of a parcel which currently has one existing building predominantly surrounded by trees with some brush. The subject site is located at the Northeast intersection of State Route 10 and County Route 112 in the Town of Caroga Lake, NY. The total parcel area is approximately 10.9 Ac with an initial project site of approximately 1.6 Ac. The property is identified by Tax Map numbers 68.17-2-1 & 68.17-2-2.

## Engineer of Record

Empire Engineering, PLLC  
1900 Duanesburg Road  
Duanesburg, NY 12056  
Contact: Christopher Longo, PE  
Phone: (518) 858-4117

## Construction Phase

### Submittals

The shop drawing design plans for all structures shall be reviewed by a NYS Licensed Professional Engineer. Specification sheets for all pipe materials and particle analyses for all aggregate to be used on site shall also be approved by the Engineer. Shop drawing and/or submittal approvals will be distributed to the owner and the contractor. No unit shall be constructed without having the Engineer's approval.

### Inspections

The Engineer shall inspect and document the installation of any structure, pipe, controlled fill and stormwater management feature. Inspections shall include documentation of the subsurface conditions and/or the soil profile including material thickness. It is the owner's responsibility to contact the engineer to witness construction. Failure to do so may result in the facility not being certified. Additional lab or field geotechnical tests may be specified by the inspecting Engineer to verify conformance with the plans. Such test would be at the owner's expense.

### Certifications

The inspecting Engineer shall issue a daily work report to the owner for each occurrence that construction is witnessed. The Engineer shall issue a letter of approval certifying stormwater components which they have witnessed and found to be in conformance with the plans, shop drawings,

and any supplemental documents. If any modifications are made to the plans or stormwater facilities the Engineer shall document such in their certification.

## Operation & Maintenance

### Recordkeeping

The owner/operation shall keep and maintain all Plans, SWPPP documents, inspection reports, and certifications generated during design and construction. These plans and reports shall be readily accessible for use by any interested party.

### Inspections

The owner should check the condition of all devices after each rainfall event for the first 30 days. Issues should be identified such as blockages or obstructions within the inlet or outlet. The owner should also inspect for accumulating sediment and conditions of slopes and embankments.

A comprehensive inspection should be completed at the end of construction in accordance with the enclosed inspection form. During operation, the owner should continue to routinely inspect all stormwater devices weekly during the rainy season. Each device should be thoroughly inspected annually. A frequency of cleaning should be determined based on the inspection findings.

### Maintenance

The owner shall maintain all stormwater devices in perpetuity. Routine maintenance should be scheduled at least annually and should address any issues identified during inspection. The enclosed maintenance checklists should be utilized for each device.

### Emergency Action Plan

In the event of an emergency condition resulting from extreme weather or a structural failure, the owner shall be contacted immediately. The local Town officials and emergency response authorities should be contacted if there is immediate danger. If the failure does not pose an immediate threat to the health or welfare of the subject adjacent properties, the engineer of record should be contacted to determine potential remedies.

## Construction Inspection Checklists

## Open Channel System Construction Inspection Checklist

Project:  
 Location:  
 Site Status:

Date:

Time:

Inspector:

CONSTRUCTION SEQUENCE	SATISFACTORY / UNSATISFACTORY	COMMENTS
<b>1. Pre-Construction</b>		
Pre-construction meeting		
Runoff diverted		
Facility location staked out		
<b>2. Excavation</b>		
Size and location		
Side slope stable		
Soil permeability		
Groundwater / bedrock		
Lateral slopes completely level		
Longitudinal slopes within design range		
Excavation does not compact subsoils		
<b>3. Check dams</b>		
Dimensions		
Spacing		
Materials		

CONSTRUCTION SEQUENCE	SATISFACTORY / UNSATISFACTORY	COMMENTS
<b>4. Structural Components</b>		
Underdrain installed correctly		
Inflow installed correctly		
Pretreatment devices installed		
<b>5. Vegetation</b>		
Complies with planting specifications		
Topsoil adequate in composition and placement		
Adequate erosion control measures in place		
<b>6. Final inspection</b>		
Dimensions		
Check dams		
Proper outlet		
Effective stand of vegetation and stabilization		
Contributing watershed stabilized before flow is routed to the facility		

Comments:

[illegible]

### Actions to be Taken:

This image shows a single sheet of white paper with horizontal blue or grey ruling lines. The lines are evenly spaced and run across the width of the page. There are approximately 20 lines visible. The paper has a slight shadow on the right side, suggesting it's resting on a surface.



## Maintenance Inspection Checklists

## Open Channel Operation, Maintenance, and Management Inspection Checklist

Project:  
Location:  
Site Status:

Date:

Time:

Inspector:

MAINTENANCE ITEM	SATISFACTORY/ UNSATISFACTORY	COMMENTS
<b>1. Debris Cleanout (Monthly)</b>		
Contributing areas clean of debris		
<b>2. Check Dams or Energy Dissipators (Annual, After Major Storms)</b>		
No evidence of flow going around structures		
No evidence of erosion at downstream toe		
Soil permeability		
Groundwater / bedrock		
<b>3. Vegetation (Monthly)</b>		
Mowing done when needed		
Minimum mowing depth not exceeded		
No evidence of erosion		
Fertilized per specification		
<b>4. Dewatering (Monthly)</b>		
Dewaters between storms		

MAINTENANCE ITEM	SATISFACTORY/ UNSATISFACTORY	COMMENTS
5. Sediment deposition (Annual)		
Clean of sediment		
6. Outlet/Overflow Spillway (Annual)		
Good condition, no need for repairs		
No evidence of erosion		

Comments:

Actions to be Taken:

## Appendix F

### Certifications



# **Owner/Operator Certification Form**

## **SPDES General Permit For Stormwater Discharges From Construction Activity (GP-0-20-001)**

**Project/Site Name:** \_\_\_\_\_

**eNOI Submission Number:** \_\_\_\_\_

**eNOI Submitted by:**                      **Owner/Operator**                      **SWPPP Preparer**                      **Other**

### **Certification Statement - Owner/Operator**

I have read or been advised of the permit conditions and believe that I understand them. I also understand that, under the terms of the permit, there may be reporting requirements. I hereby certify that this document and the corresponding documents were prepared under my direction or supervision. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations. I further understand that coverage under the general permit will be identified in the acknowledgment that I will receive as a result of submitting this NOI and can be as long as sixty (60) business days as provided for in the general permit. I also understand that, by submitting this NOI, I am acknowledging that the SWPPP has been developed and will be implemented as the first element of construction, and agreeing to comply with all the terms and conditions of the general permit for which this NOI is being submitted.

Richard  
Owner/Operator First Name

Ruby  
M.I. Last Name

\_\_\_\_\_  
Signature

\_\_\_\_\_  
Date



Department of  
Environmental  
Conservation

# SWPPP Preparer Certification Form

*SPDES General Permit for Stormwater  
Discharges From Construction Activity  
(GP-0-20-001)*

## Project Site Information

### Project/Site Name

Caroga Lake Arts Collective, Inc.

## Owner/Operator Information

### Owner/Operator (Company Name/Private Owner/Municipality Name)

Caroga Lake Arts Collective, Inc.

## Certification Statement – SWPPP Preparer

I hereby certify that the Stormwater Pollution Prevention Plan (SWPPP) for this project has been prepared in accordance with the terms and conditions of the GP-0-20-001. Furthermore, I understand that certifying false, incorrect or inaccurate information is a violation of this permit and the laws of the State of New York and could subject me to criminal, civil and/or administrative proceedings.

Christopher

First name

D

MI

Longo

Last Name

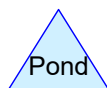
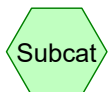
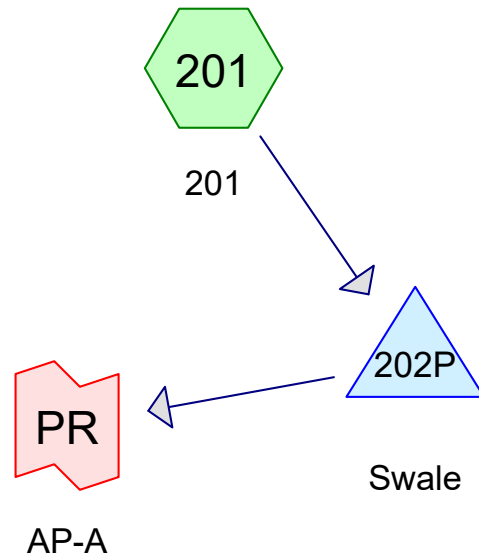
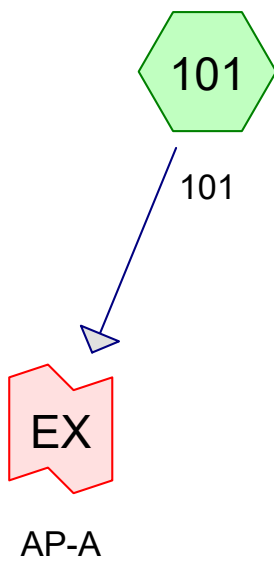
  
Signature

6/23/21

Date

## Appendix G

### Drainage Calculations





**21028 Ex HydroCAD***Type II 24-hr 1-yr Rainfall=2.20"*

Prepared by {enter your company name here}

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Page 2

Time span=5.00-36.00 hrs, dt=0.05 hrs, 621 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment101: 101**

Runoff Area=61,000 sf 9.59% Impervious Runoff Depth=0.00"  
Flow Length=300' Tc=17.4 min CN=42 Runoff=0.00 cfs 0.000 af

**Subcatchment201: 201**

Runoff Area=61,000 sf 19.26% Impervious Runoff Depth=0.00"  
Flow Length=400' Tc=18.1 min CN=48 Runoff=0.00 cfs 0.000 af

**Pond 202P: Swale**

Peak Elev=94.00' Storage=0 cf Inflow=0.00 cfs 0.000 af  
Discarded=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af

**Link EX: AP-A**

Inflow=0.00 cfs 0.000 af  
Primary=0.00 cfs 0.000 af

**Link PR: AP-A**

Inflow=0.00 cfs 0.000 af  
Primary=0.00 cfs 0.000 af

**Total Runoff Area = 2.801 ac Runoff Volume = 0.000 af Average Runoff Depth = 0.00"**  
**85.57% Pervious = 2.397 ac 14.43% Impervious = 0.404 ac**

**Summary for Subcatchment 101: 101**

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs  
Type II 24-hr 1-yr Rainfall=2.20"

Area (sf)	CN	Description
55,150	36	Woods, Fair, HSG A
* 5,850	98	Paved roads, HSG A
61,000	42	Weighted Average
55,150		90.41% Pervious Area
5,850		9.59% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.9	100	0.0500	0.10		<b>Sheet Flow, Sheet Flow</b>
					Woods: Light underbrush n= 0.400 P2= 2.80"
1.5	200	0.0200	2.28		<b>Shallow Concentrated Flow, Shallow Flow</b>
					Unpaved Kv= 16.1 fps
17.4	300	Total			

**Summary for Subcatchment 201: 201**

Runoff = 0.00 cfs @ 24.07 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs  
Type II 24-hr 1-yr Rainfall=2.20"

Area (sf)	CN	Description
49,250	36	Woods, Fair, HSG A
* 11,750	98	Paved roads, HSG A
61,000	48	Weighted Average
49,250		80.74% Pervious Area
11,750		19.26% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.9	100	0.0500	0.10		<b>Sheet Flow, Sheet Flow</b>
					Woods: Light underbrush n= 0.400 P2= 2.80"
2.2	300	0.0200	2.28		<b>Shallow Concentrated Flow, Shallow Flow</b>
					Unpaved Kv= 16.1 fps
18.1	400	Total			

**Summary for Pond 202P: Swale**

**21028 Ex HydroCAD**

Type II 24-hr 1-yr Rainfall=2.20"

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Inflow Area = 1.400 ac, 19.26% Impervious, Inflow Depth = 0.00" for 1-yr event  
 Inflow = 0.00 cfs @ 24.07 hrs, Volume= 0.000 af  
 Outflow = 0.00 cfs @ 24.14 hrs, Volume= 0.000 af, Atten= 6%, Lag= 3.8 min  
 Discarded = 0.00 cfs @ 24.14 hrs, Volume= 0.000 af  
 Primary = 0.00 cfs @ 24.14 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs  
 Peak Elev= 94.00' @ 24.14 hrs Surf.Area= 1,500 sf Storage= 0 cf

Plug-Flow detention time= 7.5 min calculated for 0.000 af (100% of inflow)  
 Center-of-Mass det. time= 7.5 min ( 1,437.2 - 1,429.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	94.00'	17,325 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
94.00	1,500	0	0
95.50	2,000	2,625	2,625
97.00	800	2,100	4,725
98.50	1,600	1,800	6,525
100.00	3,200	3,600	10,125
101.50	6,400	7,200	17,325

Device	Routing	Invert	Outlet Devices
#1	Discarded	94.00'	<b>6.000 in/hr Exfiltration over Surface area</b>
#2	Primary	94.00'	<b>2.0' long x 6.0' breadth Broad-Crested Rectangular Weir X 0.40</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.37 2.51 2.70 2.68 2.68 2.67 2.65 2.65 2.65 2.65 2.66 2.66 2.67 2.69 2.72 2.76 2.83
#3	Primary	95.50'	<b>2.0' long x 6.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.37 2.51 2.70 2.68 2.68 2.67 2.65 2.65 2.65 2.65 2.66 2.66 2.67 2.69 2.72 2.76 2.83

**Discarded OutFlow** Max=0.21 cfs @ 24.14 hrs HW=94.00' (Free Discharge)  
 ↳ **1=Exfiltration** (Exfiltration Controls 0.21 cfs)

**Primary OutFlow** Max=0.00 cfs @ 24.14 hrs HW=94.00' (Free Discharge)  
 ↳ **2=Broad-Crested Rectangular Weir** (Weir Controls 0.00 cfs @ 0.01 fps)  
 ↳ **3=Broad-Crested Rectangular Weir** ( Controls 0.00 cfs)

**Summary for Link EX: AP-A**

Inflow Area = 1.400 ac, 9.59% Impervious, Inflow Depth = 0.00" for 1-yr event  
 Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af  
 Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

**Summary for Link PR: AP-A**

Inflow Area = 1.400 ac, 19.26% Impervious, Inflow Depth = 0.00" for 1-yr event  
Inflow = 0.00 cfs @ 24.14 hrs, Volume= 0.000 af  
Primary = 0.00 cfs @ 24.14 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

**21028 Ex HydroCAD***Type II 24-hr 10-Yr Rainfall=3.50"*

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Time span=5.00-36.00 hrs, dt=0.05 hrs, 621 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment101: 101**

Runoff Area=61,000 sf 9.59% Impervious Runoff Depth=0.04"  
Flow Length=300' Tc=17.4 min CN=42 Runoff=0.01 cfs 0.004 af

**Subcatchment201: 201**

Runoff Area=61,000 sf 19.26% Impervious Runoff Depth=0.15"  
Flow Length=400' Tc=18.1 min CN=48 Runoff=0.04 cfs 0.017 af

**Pond 202P: Swale**

Peak Elev=94.01' Storage=15 cf Inflow=0.04 cfs 0.017 af  
Discarded=0.03 cfs 0.014 af Primary=0.01 cfs 0.003 af Outflow=0.03 cfs 0.017 af

**Link EX: AP-A**

Inflow=0.01 cfs 0.004 af  
Primary=0.01 cfs 0.004 af

**Link PR: AP-A**

Inflow=0.01 cfs 0.003 af  
Primary=0.01 cfs 0.003 af

**Total Runoff Area = 2.801 ac Runoff Volume = 0.021 af Average Runoff Depth = 0.09"**  
**85.57% Pervious = 2.397 ac 14.43% Impervious = 0.404 ac**

**21028 Ex HydroCAD**

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Type II 24-hr 10-Yr Rainfall=3.50"

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**Summary for Subcatchment 101: 101**

Runoff = 0.01 cfs @ 18.09 hrs, Volume= 0.004 af, Depth= 0.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs  
Type II 24-hr 10-Yr Rainfall=3.50"

Area (sf)	CN	Description
55,150	36	Woods, Fair, HSG A
* 5,850	98	Paved roads, HSG A
61,000	42	Weighted Average
55,150		90.41% Pervious Area
5,850		9.59% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.9	100	0.0500	0.10		<b>Sheet Flow, Sheet Flow</b>
					Woods: Light underbrush n= 0.400 P2= 2.80"
1.5	200	0.0200	2.28		<b>Shallow Concentrated Flow, Shallow Flow</b>
					Unpaved Kv= 16.1 fps
17.4	300	Total			

**Summary for Subcatchment 201: 201**

Runoff = 0.04 cfs @ 12.53 hrs, Volume= 0.017 af, Depth= 0.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs  
Type II 24-hr 10-Yr Rainfall=3.50"

Area (sf)	CN	Description
49,250	36	Woods, Fair, HSG A
* 11,750	98	Paved roads, HSG A
61,000	48	Weighted Average
49,250		80.74% Pervious Area
11,750		19.26% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.9	100	0.0500	0.10		<b>Sheet Flow, Sheet Flow</b>
					Woods: Light underbrush n= 0.400 P2= 2.80"
2.2	300	0.0200	2.28		<b>Shallow Concentrated Flow, Shallow Flow</b>
					Unpaved Kv= 16.1 fps
18.1	400	Total			

**Summary for Pond 202P: Swale**

**21028 Ex HydroCAD**

Type II 24-hr 10-Yr Rainfall=3.50"

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Inflow Area = 1.400 ac, 19.26% Impervious, Inflow Depth = 0.15" for 10-Yr event  
 Inflow = 0.04 cfs @ 12.53 hrs, Volume= 0.017 af  
 Outflow = 0.03 cfs @ 12.68 hrs, Volume= 0.017 af, Atten= 5%, Lag= 8.6 min  
 Discarded = 0.03 cfs @ 12.68 hrs, Volume= 0.014 af  
 Primary = 0.01 cfs @ 12.68 hrs, Volume= 0.003 af

Routing by Stor-Ind method, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs  
 Peak Elev= 94.01' @ 12.68 hrs Surf.Area= 1,503 sf Storage= 15 cf

Plug-Flow detention time= 7.7 min calculated for 0.017 af (100% of inflow)  
 Center-of-Mass det. time= 7.5 min ( 1,030.7 - 1,023.1 )

Volume	Invert	Avail.Storage	Storage Description
#1	94.00'	17,325 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
94.00	1,500	0	0
95.50	2,000	2,625	2,625
97.00	800	2,100	4,725
98.50	1,600	1,800	6,525
100.00	3,200	3,600	10,125
101.50	6,400	7,200	17,325

Device	Routing	Invert	Outlet Devices
#1	Discarded	94.00'	<b>6.000 in/hr Exfiltration over Surface area</b>
#2	Primary	94.00'	<b>2.0' long x 6.0' breadth Broad-Crested Rectangular Weir X 0.40</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.37 2.51 2.70 2.68 2.68 2.67 2.65 2.65 2.65 2.65 2.66 2.66 2.67 2.69 2.72 2.76 2.83
#3	Primary	95.50'	<b>2.0' long x 6.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.37 2.51 2.70 2.68 2.68 2.67 2.65 2.65 2.65 2.65 2.66 2.66 2.67 2.69 2.72 2.76 2.83

**Discarded OutFlow** Max=0.21 cfs @ 12.68 hrs HW=94.01' (Free Discharge)  
 ↳ **1=Exfiltration** (Exfiltration Controls 0.21 cfs)

**Primary OutFlow** Max=0.00 cfs @ 12.68 hrs HW=94.01' (Free Discharge)  
 ↳ **2=Broad-Crested Rectangular Weir** (Weir Controls 0.00 cfs @ 0.10 fps)  
 ↳ **3=Broad-Crested Rectangular Weir** ( Controls 0.00 cfs)

**Summary for Link EX: AP-A**

Inflow Area = 1.400 ac, 9.59% Impervious, Inflow Depth = 0.04" for 10-Yr event  
 Inflow = 0.01 cfs @ 18.09 hrs, Volume= 0.004 af  
 Primary = 0.01 cfs @ 18.09 hrs, Volume= 0.004 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

**Summary for Link PR: AP-A**

Inflow Area = 1.400 ac, 19.26% Impervious, Inflow Depth = 0.02" for 10-Yr event  
Inflow = 0.01 cfs @ 12.68 hrs, Volume= 0.003 af  
Primary = 0.01 cfs @ 12.68 hrs, Volume= 0.003 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs



**21028 Ex HydroCAD***Type II 24-hr 100-Yr Rainfall=5.80"*

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Time span=5.00-36.00 hrs, dt=0.05 hrs, 621 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment101: 101**

Runoff Area=61,000 sf 9.59% Impervious Runoff Depth=0.55"  
Flow Length=300' Tc=17.4 min CN=42 Runoff=0.44 cfs 0.064 af

**Subcatchment201: 201**

Runoff Area=61,000 sf 19.26% Impervious Runoff Depth=0.91"  
Flow Length=400' Tc=18.1 min CN=48 Runoff=1.09 cfs 0.106 af

**Pond 202P: Swale**

Peak Elev=94.35' Storage=552 cf Inflow=1.09 cfs 0.106 af  
Discarded=0.22 cfs 0.075 af Primary=0.42 cfs 0.031 af Outflow=0.64 cfs 0.106 af

**Link EX: AP-A**

Inflow=0.44 cfs 0.064 af  
Primary=0.44 cfs 0.064 af

**Link PR: AP-A**

Inflow=0.42 cfs 0.031 af  
Primary=0.42 cfs 0.031 af

**Total Runoff Area = 2.801 ac Runoff Volume = 0.170 af Average Runoff Depth = 0.73"**  
**85.57% Pervious = 2.397 ac 14.43% Impervious = 0.404 ac**

**21028 Ex HydroCAD**

Type II 24-hr 100-Yr Rainfall=5.80"

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**Summary for Subcatchment 101: 101**

Runoff = 0.44 cfs @ 12.17 hrs, Volume= 0.064 af, Depth= 0.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs  
Type II 24-hr 100-Yr Rainfall=5.80"

Area (sf)	CN	Description
55,150	36	Woods, Fair, HSG A
* 5,850	98	Paved roads, HSG A
61,000	42	Weighted Average
55,150		90.41% Pervious Area
5,850		9.59% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.9	100	0.0500	0.10		<b>Sheet Flow, Sheet Flow</b>
					Woods: Light underbrush n= 0.400 P2= 2.80"
1.5	200	0.0200	2.28		<b>Shallow Concentrated Flow, Shallow Flow</b>
					Unpaved Kv= 16.1 fps
17.4	300	Total			

**Summary for Subcatchment 201: 201**

Runoff = 1.09 cfs @ 12.15 hrs, Volume= 0.106 af, Depth= 0.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs  
Type II 24-hr 100-Yr Rainfall=5.80"

Area (sf)	CN	Description
49,250	36	Woods, Fair, HSG A
* 11,750	98	Paved roads, HSG A
61,000	48	Weighted Average
49,250		80.74% Pervious Area
11,750		19.26% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.9	100	0.0500	0.10		<b>Sheet Flow, Sheet Flow</b>
					Woods: Light underbrush n= 0.400 P2= 2.80"
2.2	300	0.0200	2.28		<b>Shallow Concentrated Flow, Shallow Flow</b>
					Unpaved Kv= 16.1 fps
18.1	400	Total			

**Summary for Pond 202P: Swale**

**21028 Ex HydroCAD**

Type II 24-hr 100-Yr Rainfall=5.80"

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Inflow Area = 1.400 ac, 19.26% Impervious, Inflow Depth = 0.91" for 100-Yr event  
 Inflow = 1.09 cfs @ 12.15 hrs, Volume= 0.106 af  
 Outflow = 0.64 cfs @ 12.32 hrs, Volume= 0.106 af, Atten= 41%, Lag= 10.5 min  
 Discarded = 0.22 cfs @ 12.32 hrs, Volume= 0.075 af  
 Primary = 0.42 cfs @ 12.32 hrs, Volume= 0.031 af

Routing by Stor-Ind method, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs  
 Peak Elev= 94.35' @ 12.32 hrs Surf.Area= 1,618 sf Storage= 552 cf

Plug-Flow detention time= 9.8 min calculated for 0.106 af (100% of inflow)  
 Center-of-Mass det. time= 9.8 min ( 925.7 - 915.9 )

Volume	Invert	Avail.Storage	Storage Description
#1	94.00'	17,325 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
94.00	1,500	0	0
95.50	2,000	2,625	2,625
97.00	800	2,100	4,725
98.50	1,600	1,800	6,525
100.00	3,200	3,600	10,125
101.50	6,400	7,200	17,325

Device	Routing	Invert	Outlet Devices
#1	Discarded	94.00'	<b>6.000 in/hr Exfiltration over Surface area</b>
#2	Primary	94.00'	<b>2.0' long x 6.0' breadth Broad-Crested Rectangular Weir X 0.40</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.37 2.51 2.70 2.68 2.68 2.67 2.65 2.65 2.65 2.65 2.66 2.66 2.67 2.69 2.72 2.76 2.83
#3	Primary	95.50'	<b>2.0' long x 6.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.37 2.51 2.70 2.68 2.68 2.67 2.65 2.65 2.65 2.65 2.66 2.66 2.67 2.69 2.72 2.76 2.83

**Discarded OutFlow** Max=0.22 cfs @ 12.32 hrs HW=94.35' (Free Discharge)  
 ↳ **1=Exfiltration** (Exfiltration Controls 0.22 cfs)

**Primary OutFlow** Max=0.41 cfs @ 12.32 hrs HW=94.35' (Free Discharge)  
 ↳ **2=Broad-Crested Rectangular Weir** (Weir Controls 0.41 cfs @ 0.59 fps)  
 ↳ **3=Broad-Crested Rectangular Weir** ( Controls 0.00 cfs)

**Summary for Link EX: AP-A**

Inflow Area = 1.400 ac, 9.59% Impervious, Inflow Depth = 0.55" for 100-Yr event  
 Inflow = 0.44 cfs @ 12.17 hrs, Volume= 0.064 af  
 Primary = 0.44 cfs @ 12.17 hrs, Volume= 0.064 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

**Summary for Link PR: AP-A**

Inflow Area = 1.400 ac, 19.26% Impervious, Inflow Depth = 0.27" for 100-Yr event  
Inflow = 0.42 cfs @ 12.32 hrs, Volume= 0.031 af  
Primary = 0.42 cfs @ 12.32 hrs, Volume= 0.031 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

**21028 Ex HydroCAD***Type II 24-hr WQv Rainfall=1.00"*

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Time span=5.00-36.00 hrs, dt=0.05 hrs, 621 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment101: 101**

Runoff Area=61,000 sf 9.59% Impervious Runoff Depth=0.00"  
Flow Length=300' Tc=17.4 min CN=42 Runoff=0.00 cfs 0.000 af

**Subcatchment201: 201**

Runoff Area=61,000 sf 19.26% Impervious Runoff Depth=0.00"  
Flow Length=400' Tc=18.1 min CN=48 Runoff=0.00 cfs 0.000 af

**Pond 202P: Swale**

Peak Elev=94.00' Storage=0 cf Inflow=0.00 cfs 0.000 af  
Discarded=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af

**Link EX: AP-A**

Inflow=0.00 cfs 0.000 af  
Primary=0.00 cfs 0.000 af

**Link PR: AP-A**

Inflow=0.00 cfs 0.000 af  
Primary=0.00 cfs 0.000 af

**Total Runoff Area = 2.801 ac Runoff Volume = 0.000 af Average Runoff Depth = 0.00"**  
**85.57% Pervious = 2.397 ac 14.43% Impervious = 0.404 ac**

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Type II 24-hr WQv Rainfall=1.00"

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**Summary for Subcatchment 101: 101**

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs  
Type II 24-hr WQv Rainfall=1.00"

Area (sf)	CN	Description
55,150	36	Woods, Fair, HSG A
* 5,850	98	Paved roads, HSG A
61,000	42	Weighted Average
55,150		90.41% Pervious Area
5,850		9.59% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.9	100	0.0500	0.10		<b>Sheet Flow, Sheet Flow</b>
					Woods: Light underbrush n= 0.400 P2= 2.80"
1.5	200	0.0200	2.28		<b>Shallow Concentrated Flow, Shallow Flow</b>
					Unpaved Kv= 16.1 fps
17.4	300	Total			

**Summary for Subcatchment 201: 201**

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs  
Type II 24-hr WQv Rainfall=1.00"

Area (sf)	CN	Description
49,250	36	Woods, Fair, HSG A
* 11,750	98	Paved roads, HSG A
61,000	48	Weighted Average
49,250		80.74% Pervious Area
11,750		19.26% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.9	100	0.0500	0.10		<b>Sheet Flow, Sheet Flow</b>
					Woods: Light underbrush n= 0.400 P2= 2.80"
2.2	300	0.0200	2.28		<b>Shallow Concentrated Flow, Shallow Flow</b>
					Unpaved Kv= 16.1 fps
18.1	400	Total			

**Summary for Pond 202P: Swale**

**21028 Ex HydroCAD**

Type II 24-hr WQv Rainfall=1.00"

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Inflow Area = 1.400 ac, 19.26% Impervious, Inflow Depth = 0.00" for WQv event  
 Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af  
 Outflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min  
 Discarded = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af  
 Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs  
 Peak Elev= 94.00' @ 5.00 hrs Surf.Area= 1,500 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)  
 Center-of-Mass det. time= (not calculated: no inflow)

Volume	Invert	Avail.Storage	Storage Description
#1	94.00'	17,325 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
94.00	1,500	0	0
95.50	2,000	2,625	2,625
97.00	800	2,100	4,725
98.50	1,600	1,800	6,525
100.00	3,200	3,600	10,125
101.50	6,400	7,200	17,325

Device	Routing	Invert	Outlet Devices
#1	Discarded	94.00'	<b>6.000 in/hr Exfiltration over Surface area</b>
#2	Primary	94.00'	<b>2.0' long x 6.0' breadth Broad-Crested Rectangular Weir X 0.40</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.37 2.51 2.70 2.68 2.68 2.67 2.65 2.65 2.65 2.65 2.66 2.66 2.67 2.69 2.72 2.76 2.83
#3	Primary	95.50'	<b>2.0' long x 6.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.37 2.51 2.70 2.68 2.68 2.67 2.65 2.65 2.65 2.65 2.66 2.66 2.67 2.69 2.72 2.76 2.83

**Discarded OutFlow** Max=0.00 cfs @ 5.00 hrs HW=94.00' (Free Discharge)  
 ↳ **1=Exfiltration** (Passes 0.00 cfs of 0.21 cfs potential flow)

**Primary OutFlow** Max=0.00 cfs @ 5.00 hrs HW=94.00' (Free Discharge)  
 ↳ **2=Broad-Crested Rectangular Weir** ( Controls 0.00 cfs)  
 ↳ **3=Broad-Crested Rectangular Weir** ( Controls 0.00 cfs)

**Summary for Link EX: AP-A**

Inflow Area = 1.400 ac, 9.59% Impervious, Inflow Depth = 0.00" for WQv event  
 Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af  
 Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

**Summary for Link PR: AP-A**

Inflow Area = 1.400 ac, 19.26% Impervious, Inflow Depth = 0.00" for WQv event  
Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af  
Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs



## Appendix H

### Water Quality Worksheets

Is this project subject to Chapter 10 of the NYS Design Manual (i.e. WQv is equal to post-development 1 year runoff volume)?.....

No

Design Point: A

P=

1.15

inch

*Manually enter P, Total Area and Impervious Cover.***Breakdown of Subcatchments**

Catchment Number	Total Area (Acres)	Impervious Area (Acres)	Percent Impervious %	Rv	WQv (ft <sup>3</sup> )	Description
1	1.20	0.16	13%	0.17	852	Dry Swale
2						
3						
4						
5						
6						
7						
8						
9						
10						
Subtotal (1-30)	1.20	0.16	13%	0.17	852	Subtotal 1
<b>Total</b>	1.20	0.16	13%	0.17	852	<b>Initial WQv</b>

**Identify Runoff Reduction Techniques By Area**

Technique	Total Contributing Area	Contributing Impervious Area	Notes
	(Acre)	(Acre)	
Conservation of Natural Areas	0.00	0.00	minimum 10,000 sf
Riparian Buffers	0.00	0.00	maximum contributing length 75 feet to 150 feet
Filter Strips	0.00	0.00	
Tree Planting	0.00	0.00	Up to 100 sf directly connected impervious area may be subtracted per tree
<b>Total</b>	<b>0.00</b>	<b>0.00</b>	

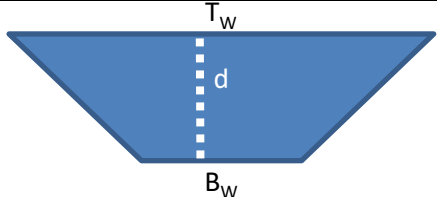
**Recalculate WQv after application of Area Reduction Techniques**

	Total Area (Acres)	Impervious Area (Acres)	Percent Impervious %	Runoff Coefficient Rv	WQv (ft <sup>3</sup> )
"<<Initial WQv"	1.20	0.16	13%	0.17	852
Subtract Area	0.00	0.00			
WQv adjusted after Area Reductions	<b>1.20</b>	<b>0.16</b>	13%	0.17	852
Disconnection of Rooftops		0.00			
Adjusted WQv after Area Reduction and Rooftop Disconnect	1.20	0.16	13%	0.17	<b>852</b>
WQv reduced by Area Reduction techniques					0

Minimum RRv

Enter the Soils Data for the site		
Soil Group	Acres	S
A		55%
B		40%
C	1.20	30%
D		20%
Total Area	1.2	
Calculate the Minimum RRv		
S =	0.30	
Impervious =	0.16	acre
Precipitation	1.15	in
Rv	0.95	
Minimum RRv	190	ft3
	0.00	af

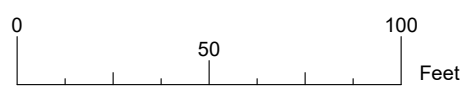
# Dry Swale Worksheet

<b>Design Point:</b>	A						
<b>Enter Site Data For Drainage Area to be Treated by Practice</b>							
Catchment Number	Total Area (Acres)	Impervious Area (Acres)	Percent Impervious %	Rv	WQv (ft <sup>3</sup> )	Precipitation (in)	Description
1	1.20	0.16	0.13	0.17	851.60	1.15	Dry Swale
Enter Impervious Area Reduced by Disconnection of Rooftops		0.00	13%	0.17	852	<<WQv after adjusting for Disconnected Rooftops	
<b>Pretreatment Provided</b>					<b>Pretreatment Technique</b>		
Pretreatment (10% of WQv)			85	ft <sup>3</sup>	100		
<b>Calculate Available Storage Capacity</b>							
Bottom Width	2	ft	Design with a bottom width no greater than eight feet to avoid potential gullyng and channel braiding, but no less than two feet				
Side Slope (X:1)	3	Okay	Channels shall be designed with moderate side slopes (flatter than 3:1) for most conditions. 2:1 is the absolute maximum side slope				
Longitudinal Slope	1%	Okay	Maximum longitudinal slope shall be 4%				
Flow Depth	0.75	ft	Maximum ponding depth of one foot at the mid-point of the channel, and a maximum depth of 18" at the end point of the channel (for storage of the WQv)				
Top Width	6.5	ft					
Area	3.19	sf					
Minimum Length	240	ft					
Actual Length	288	ft					
End Point Depth check	1.50	Okay	A maximum depth of 18" at the end point of the channel (for storage of the WQv)				
Storage Capacity	1,003	ft <sup>3</sup>					
Soil Group (HSG)			C				
<b>Runoff Reduction</b>							
Is the Dry Swale contributing flow to another practice?			No	Select Practice	N/A		
<b>RRv</b>	<b>201</b>	<b>ft<sup>3</sup></b>	<b>Runnoff Reduction equals 40% in HSG A and B and 20% in HSG C and D up to the WQv</b>				
Volume Treated	651	ft <sup>3</sup>	This is the difference between the WQv calculated and the runoff reduction achieved in the swale				
Volume Directed	0	ft <sup>3</sup>	This volume is directed another practice				
Volume V	Okay		Check to be sure that channel is long enough to store WQv				

Appendix I

Project Plan Sheets





PROJECT INFORMATION:

PROPERTY TAX MAP NUMBER  
68.17-2-1 & 68.17-2-2

PARCEL AREA:

ZONING:  
H-HAMLET

MAP REFERENCE

NOTES:

- 1) SURVEYED PARCEL: TOWN OF DUANESBURG – TAX MAP 74.00, BLOCK 1, PARCEL 27.1.

2) BASE SURVEY PREPARED BY TOWN OF CAROGA, FROM AN OCTOBER 2020 FIELD SURVEY.

3) NORTH IS REFERENCED TO NAD 83 NEW YORK STATE PLANES EAST ZONE. ELEVATIONS

4) SUBJECT TO ALL RIGHTS, EASEMENTS, COVENANTS OR RESTRICTION; RECORDED OR UNRECORDED.

5) SUBJECT TO ANY STATEMENT OF FACT CONTAINED IN AN UP TO DATE ABSTRACT OF TITLE OR TITLE REPORT.

6) UNDERGROUND UTILITIES IF SHOWN HEREON ARE BASED ON VISIBLE PHYSICAL EVIDENCE. THEY SHOULD BE CONSIDERED SCHEMATIC ONLY AND ARE SHOWN TO DEPICT GENERAL UTILITY CONNECTIONS RATHER THAN EXACT UNDERGROUND LOCATIONS. GERALD R GRAY PLS MAKES NO CERTIFICATION AS TO THE ACCURACY OF THE UNDERGROUND UTILITY LOCATIONS AND OTHER UTILITIES MAY EXIST THAT ARE NOT SHOWN ON THIS MAP. CALL DIGSAFE FOR FURTHER VERIFICATION.

7) SURVEY IS PREPARED IN ACCORDANCE WITH THE NEW YORK STATE ASSOCIATION OF PROFESSIONAL LAND SURVEYORS CODE OF PRACTICE FOR LAND SURVEYS AS ADOPTED IN OCTOBER OF 1966 AND LAST REVISED ON JULY 18, 1997.

EROSION & SEDIMENT CONTROL GENERAL  
NOTES:

1. THIS PROJECT QUALIFIES FOR COVERAGE UNDER THE NYSDEC GENERAL PERMIT GP 0-20-001. A NOTICE OF INTENT (NOI) MUST BE FILED WITH THE NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION AND AUTHORIZATION RECEIVED PRIOR TO CONSTRUCTION ACTIVITIES.
2. A STORMWATER POLLUTION PREVENTION PLAN HAS BEEN PREPARED IN ACCORDANCE WITH THE NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION SEDIMENT EROSION PERMIT FOR STORMWATER DISCHARGES GP 0-20-001 AND SHALL BE CONSIDERED SUPPLEMENTAL TO THESE PLANS.
3. ANY CONTRACTOR INVOLVED IN ANY EARTHWORK ACTIVITY SHALL REVIEW ALL PLANS AND PERMIT CONDITIONS AND CERTIFY ACKNOWLEDGEMENT IN WRITING. IT IS THE CONTRACTOR'S RESPONSIBILITY TO IMPLEMENT ALL EROSION CONTROLS DESCRIBED IN GP 0-20-001, AND IT IS NOT THE INTENT OF THESE DRAWINGS TO REPLACE OR DISSEMINATE THE PERMIT REQUIREMENTS. THE CONTRACTOR SHALL REMAIN IN COMPLIANCE WITH THE PERMIT AT ALL TIMES.
4. NO MORE THAN FIVE (5) ACRES OF SOIL SHALL BE DISTURBED AT ONE TIME. THE CONTRACTOR SHALL COORDINATE EARTHWORK ACTIVITIES AND IMPLEMENTATION OF SOIL STABILIZATION MEASURES TO ENSURE COMPLIANCE WITH THIS REQUIREMENT.
5. DISTURBED AREAS SHALL NOT BE LEFT UNSTABILIZED FOR MORE THAN 14 DAYS AFTER COMPLETION OR SUSPENSION OF GRADING OPERATIONS.
6. EROSION AND SEDIMENT CONTROL MEASURES SHALL BE IMPLEMENTED IN ACCORDANCE WITH THE LATEST EDITION OF NEW YORK STATE STANDARDS AND SPECIFICATIONS FOR EROSION AND SEDIMENT CONTROL." (see the BLUE BOOK) EROSION CONTROL DEVICES SHALL BE INSTALLED PRIOR TO ANY CONSTRUCTION ACTIVITIES.
7. EROSION CONTROL DEVICES SHALL NOT BE REMOVED UNTIL THE TOWN ENGINEER HAS APPROVED FINAL STABILIZATION.
8. SILT FENCE AND OTHER EROSION CONTROL DEVICES SHALL BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH THESE DETAILS SHEETS AND SECTION 7A OF THE BLUE BOOK.
9. PRIOR TO ANY CONSTRUCTION ACTIVITY, THE STABILIZED CONSTRUCTION ENTRANCES SHALL BE INSTALLED.

TEMPORARY EROSION AND SEDIMENT CONTROL NOTES:

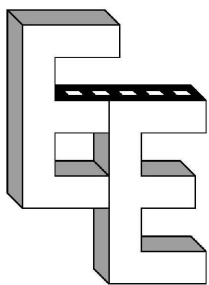
1. IT IS THE INTENT OF THESE PLANS AND NOTES TO BE USED AS A GUIDE BY THE CONTRACTOR TO ENSURE THAT NO ERODED MATERIAL MIGRATES FROM THE SITE OR ENERS ANY WATER COURSE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT THE MATERIAL IS MET, BY IMPLEMENTING THESE PLANS AND ANY ADDITIONAL MEANS THAT MAY BE NECESSARY. FURTHER MEASURES MAY BE REQUIRED BY THE CITY, VILLAGE, OR TOWN ENGINEER. WHILE MANY OF THE EROSION CONTROL DETAILS CONTAINED WITHIN THESE PLANS ARE SUBJECT TO APPROVAL BY THE BLUE BOOK, THE CONTRACTOR SHOULD CONSIDER ANY OF THE DETAILS CONTAINED IN SECTION 7A OF THE BLUE BOOK AN ACCEPTABLE PRACTICE IN THE APPROPRIATE APPLICATION.
2. THE OWNER/OPERATOR SHALL INSPECT AND MAINTAIN EROSION CONTROL MEASURES DAILY AND AFTER EACH RAINFALL EVENT THROUGH THE ENTIRE DEVELOPMENT PROCESS. TO ASSURE PROPER EROSION CONTROL, THE SLOPE SHOULD BE MAINTAINED IN GOOD CONDITION AND REINFORCED, EXTENDED, REPAIRED, RE-SEEDED AND PROTECTED FROM FURTHER EROSION. ALL SEDIMENT ACCUMULATED SHALL BE REMOVED AND CONTAINED IN APPROPRIATE SPOIL AREAS. THE SLOPE SHOULD BE REPAIRED TO THE NEWLY SEED AREAS. UNTIL GRASS COVER IS WELL ESTABLISHED DURING THESE PERIODIC INSPECTIONS, THE FOLLOWING ITEMS SHOULD BE PAID PARTICULAR ATTENTION: A. SILT FENCING SHALL BE INSPECTED FOR UNDERMINING AND DETEIORATION. B. SEEDED/MULCHED AREAS SHALL BE INSPECTED TO SEE THAT A GOOD STAND IS MAINTAINED. AREAS SHALL BE REPAIRED AS NECESSARY.
3. PRIOR TO ANY CONSTRUCTION ACTIVITY, THE STABILIZED CONSTRUCTION ENTRANCES SHALL BE INSTALLED.
4. SILT FENCE AND OTHER EROSION CONTROL DEVICES SHALL BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH THESE DETAILS. SHEETS 10-1 AND 10-2 OF THE BLUE BOOK. IF FENCING SHORTS BE INSTALLED AT THE PERIMETER OF ALL SLOPES TO BE GRADED, PRIOR TO GRADING OPERATIONS.
5. CLEARING OPERATIONS SHALL BE LIMITED TO ACTIVE WORK AREAS.
6. CARE SHALL BE TAKEN TO PRESERVE AS MUCH EXISTING VEGETATION AS POSSIBLE. AND HEALTHY TREES OF DESIRABLE SPECIES SHALL BE PROTECTED.
7. CONSTRUCTION TRAFFIC SHALL NOT CROSS STREAMS OR DITCHES. ANYCUT AT ADJACENT CROSSING FACILITIES EQUIPMENT SHALL NOT OPERATE, UNNECESSARILY, WITHIN WATERSHEDS OR DRAINAGE DITCHES.
8. EXISTING PAVEMENT AREAS SHALL BE CLEANED AT THE DIRECTION OF THE CITY, VILLAGE, OR TOWN ENGINEER.
9. WATER TRUCKS SHALL BE USED TO MINIMIZE DUST POLLUTION ON CITY STREETS AND IN ADJACENT PAVED AREAS AS DIRECTED BY THE CITY, VILLAGE, OR TOWN ENGINEER.
10. WATER PUMPED AS A RESULT OF DEWATERING ACTIVITIES SHALL BE PUMPED INTO A DEWATERING PIT.
11. ALL AREAS DISTURBED IN THE CONSTRUCTION PROCESS SHALL BE STABILIZED WITH SEED AND MULCH NO MORE THAN 14 DAYS AFTER THE COMPLETION OF WORK. IN SUCH AREA IT MAY BE NECESSARY TO SEED AND MULCH SOME AREAS SEVERAL TIMES TO MEET THIS REQUIREMENT.
12. ALL EROSION CONTROL DEVICES SHALL BE PLACED IN THE ENTIRE PHASE AS SHOWN ON THE EROSION CONTROL PLAN PRIOR TO ANY WORK ON SUCH PHASE.



PRIOR TO ANY EARTH  
TURBANCE THE CONTRACTOR  
MAY CALL IN A TICKET TO  
GET SAFE NY AND OBTAIN A  
CLEAR TO DIG

IT IS A VIOLATION OF SECTION 7209 OF THE NYS EDUCATION LAW FOR ANY PERSON TO ALTER ANY ITEM ON THIS PLAN IN ANY WAY UNLESS HE/SHE IS ACTING UNDER THE DIRECT SUPERVISION OF A PROFESSIONAL ENGINEER.

CHRISTOPHER D. LONGO, PE  
N.Y.S. LIC. # 095840



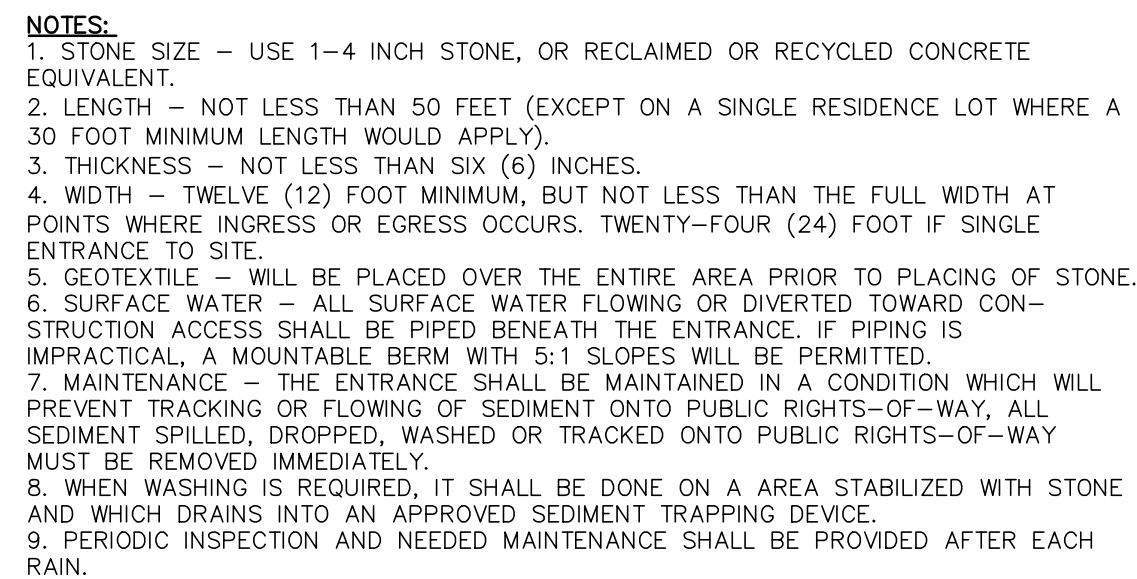
EMPIRE ENGINEERING, PLLC  
1900 DUANESBURG ROAD  
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PROJECT  
CAROGA LAKE ARTS  
COLLECTIVE, INC.

1989 STATE HWY 10  
CAROGA LAKE, NY

Title E&SC PLAN	
Date 07/21/2021	Sheet C101
Scale 1"=40'	
Job# 21028	Sheet 1 of 2



[illegible]

**NOTES:**

1. WOVEN WIRE FENCE TO BE FASTENED SECURELY TO FENCE POSTS WITH WIRE TIES OR STAPLES. POSTS SHALL BE STEEL EITHER "T" OR "U" TYPE OR HARDWOOD.
2. FILTER CLOTH TO BE TO BE FASTENED SECURELY TO WOVEN WIRE FENCE WITH TIES SPACED EVERY 24" AT TOP AND MID SECTION. FENCE SHALL BE WOVEN WIRE, 6" MAXIMUM MESH OPENING.
3. WHEN TWO SECTIONS OF FILTER CLOTH ADJOIN EACH OTHER THEY SHALL BE OVERLAPPED BY SIX INCHES AND FOLDED. FILTER CLOTH SHALL BE EITHER FILTER X, MIRAFIL 100X, STABILINKA T140N, OR APPROVED EQUIVALENT.
4. IMPROVED ERICATED UNIT SHALL BE GEOPAR, ENVIROFENCE, OR APPROVED EQUIVALENT.
5. MAINTENANCE SHALL BE PERFORMED AS NEEDED AND MATERIAL REMOVED WHEN "BULGES" DEVELOP IN THE SILT FENCE.

A cross-sectional diagram of a topsoil stockpile. The stockpile is a mound of soil with a slope indicated by a triangle with a vertical side of 2 and a horizontal side of 1, labeled "SLOPE OR LESS". The top surface is labeled "TOPSOIL STOCKPILE" and is covered with small downward-pointing arrows. A "SILT FENCE (SURROUNDING ENTIRE PILE)" is shown as a series of vertical posts around the base of the stockpile. The area between the stockpile and the silt fence is labeled "SLOPE GROUND AWAY FROM PILE". At the bottom center, there is an "ENTRANCE 24' WIDE AND SHALL BE ON UPSLOPE". This entrance is flanked by two areas labeled "SLOPE GROUND AWAY FROM PILE". A "SILT FENCE OR STRAW BAILS" is shown as a series of vertical posts across the entrance.

- NOTES:**
1. SILT FENCE SHALL BE INSTALLED PER DETAIL.
  2. IF THE STOCKPILE IS TO REMAIN FOR MORE THAN 14 DAYS, IT SHALL BE STABILIZED WITH SEED AND MULCH IMMEDIATELY AFTER COMPLETION OF STOCKPILING.
  3. SILT FENCE SHALL BE INSPECTED WEEKLY AND SEDIMENT TRAPPED BY THE FENCING SHALL BE REMOVED OF AS NECESSARY.
  4. SILT FENCE SHALL REMAIN IN PLACE UNTIL THE ENTIRE PILE OF MATERIAL HAS BEEN ELIMINATED.
  5. AREA CHOSEN FOR STOCKPILING OPERATIONS SHALL BE DRY AND STABLE.

The figure illustrates the design of a riprap apron. The top part is a plan view showing a rectangular area with dimensions: 1' MIN. width, 1' MAX. length, and 4' MIN. width. It also shows the 'TOP OF EMBANKMENT' and 'EXISTING GROUND' levels. The bottom part is a cross-section A-A showing the apron structure with a 'WEIR CREST', 'SMALL RIPRAP', 'EARTH EMBANKMENT', and '2' APRON'. It also indicates 'N.Y.S. DOT #2 STONE (OPTIONAL)' and 'UNDISTURBED AREA'.

- NOTES:**
1. AREA UNDER EMBANKMENT SHALL BE CLEARED, GRUBBED AND STRIPPED OF ANY VEGETATION AND ROOT MAT. THE POOL AREA SHALL BE CLEARED.
  2. VEGETATION FOR THE EMBANKMENT SHALL BE FREE OF ROOTS AND OTHER WOODY VEGETATION AS WELL AS OVER-SIZED STONES, ROCKS, ORGANIC MATERIAL OR OTHER OBJECTIONABLE MATERIAL. THE EMBANKMENT SHALL BE COMPACTED BY TRAVERSING WITH EQUIPMENT WHILE IT IS BEING CONSTRUCTED.
  3. THE CUT AND FILL SLOPES SHALL BE 2:1 OR FLATTER.
  4. THE STONE USED IN THE OUTLET SHALL BE SMALL RIPRAP 4"-8" ALONG WITH A 1' THICKNESS OF 2" AGGREGATE PLACED ON THE UP-GRADE SIDE ON THE SMALL RIPRAP OR EMBEDDED FILTER CLOTH IN THE RIPRAP.
  5. SEDIMENT SHALL BE REMOVED AND TRAP RESTORED TO ITS ORIGINAL DIMENSIONS WHEN THE SEDIMENT HAS ACCUMULATED TO 1/2 THE DESIGN DEPTH OF THE TRAP.
  6. THE STRUCTURE SHALL BE INSPECTED AFTER EACH RAIN AND REPAIRS MADE AS NEEDED.
  7. CONSTRUCTION OPERATIONS SHALL BE CARRIED OUT IN SUCH A MANNER THAT EROSION AND WATER POLLUTION IS MINIMIZED.
  8. THE STRUCTURE SHALL BE REMOVED AND THE AREA STABILIZED WHEN THE DRAINAGE AREA HAS BEEN PROPERLY STABILIZED.
- MAXIMUM DRAINAGE AREA 5 ACRES

PLAN

SECTION A

### CONCRETE WASHOUT AREA INSTALLATION NOTES

1. SEE PLAN VIEW FOR LOCATIONS OF CONCRETE WASHOUT AREA
2. THE CONCRETE WASHOUT AREA SHALL BE INSTALLED PRIOR TO ANY CONCRETE PLACEMENT ON SITE.
3. VEHICLE TRACKING CONTROL IS REQUIRED AT THE ACCESS POINT.
4. SIGNS SHALL BE PLACED AT THE CONSTRUCTION ENTRANCE, AT THE WASHOUT AREA, AND ELSEWHERE AS NECESSARY TO CLEARLY INDICATE THE LOCATION OF CONCRETE WASHOUT AREA TO ALL OPERATORS OF CONCRETE TRUCKS AND MIXERS.
5. A POLYETHYLENE LINER MINIMUM 16 MIL THICKNESS SHALL BE INSTALLED AND SECURED WITHIN THE WASHOUT AREA.
6. EXCAVATED MATERIAL SHALL BE UTILIZED IN PERIMETER BERM CONSTRUCTION.

### CONCRETE WASHOUT AREA MAINTENANCE NOTES

1. THE CONCRETE WASHOUT AREA SHALL BE REPAIRED AND ENLARGED OR CLEANED OUT AS NECESSARY TO MAINTAIN CAPACITY FOR WASTED CONCRETE.
2. AT THE END OF CONSTRUCTION, ALL CONCRETE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF AT AN APPROVED WASTE SITE.
3. WHEN THE CONCRETE WASHOUT AREA IS REMOVED, COVER THE DISTURBED AREA WITH TOPSOIL, SEED AND MULCH OR OTHERWISE STABILIZE IN A MANNER APPROVED BY THE LOCAL JURISDICTION.
4. INSPECT WEEKLY, DURING AND AFTER ANY STORM EVENT.

The drawings illustrate the design of a ditch and cutoff trench. The plan view at the top shows a ditch with a 1.5' minimum width and a 9' minimum length. The cross-section (Section A-A) shows a ditch with a 24" maximum height at the center and a 6" height at the toe. The detail of the trench bottom (Section B-B) shows a 18" wide trench with a 6" height at the toe and a 24" maximum height at the center. The drawings also indicate the placement of filter fabric and the design bottom of the cutoff trench.

SPACING VARIES DEPENDING ON CHANNEL SLOPE

X

CUTOFF TRENCH  
18" WIDE  
6" DEEP

TOE

SLOPE

A

CREST  
24" MAX  
@ CENTER

H

A

PROFILE  
NOT TO SCALE

GROUND LINE

1.5' MIN.

9' MIN.

B

B

FILTER FABRIC

DITCH BOTTOM  
CUTOFF TRENCH  
DESIGN BOTTOM

SECTION A-A  
NOT TO SCALE

$$X = \frac{H (Ft)}{\text{SLOPE (Ft/Ft)}}$$

24" MAX  
@ CENTER

6"

18"

FILTER FABRIC

SECTION B-B  
NOT TO SCALE

- NOTES:

1. STONE WILL BE PLACED ON A FILTER FABRIC FOUNDATION TO THE LINES, GRADES AND LOCATIONS SHOWN IN THE PLAN.
2. SET SPACING OF CHECK DAMS TO ASSUME THAT THE ELEVATIONS OF THE CREST OF THE DOWNSTREAM DAM IS 10 FEET ABOVE ELEVATION OF STONE TOP OF THE UPSTREAM DAM.
3. EXTEND THE STONE A MINIMUM OF 1.5 FEET BEYOND THE DITCH BANKS TO PREVENT CUTTING AROUND THE DAM.
4. PROTECT THE CHANNEL DOWNSTREAM OF THE LOWEST CHECK DAM FROM SCOUR AND EROSION WITH STONE OR LINER AS APPROPRIATE.
5. ENSURE THAT CHANNEL APPURTENANCES SUCH AS CULVERT ENTRANCES BELOW CHECK DAMS ARE NOT SUBJECT TO DAMAGE OR BLOCKAGE FROM DISPLACED STONE.
6. MAXIMUM DRAINAGE AREA 2 ACRES.

GRASS VEGETATION

2'

3'

1'

2'

9"

1'

3'

RIP RAP  
CHECK DAM  
SEE PLAN  
FOR LOCATION

NON-WOVEN  
GEOTEXTILE FILTER  
FABRIC

18" PERMEABLE  
SAND FILTER MEDIA

6" WASHED  
GRAVEL

6" PERFORATED  
UNDERDRAIN

2' MIN. TO GROUNDWATER

- NOTES

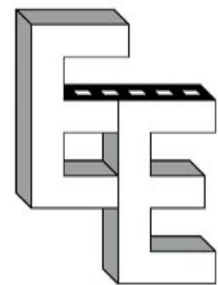
1. DISTURBED AREAS SHALL BE PLANTED WITH ENVIRONMENTAL SEED MIX.
2. VEGETATION SHALL BE MAINTAINED AT 6" HEIGHT.
3. ALL TREES, BRUSH, STUMPS AND OTHER OBSTRUCTIONS SHALL BE REMOVED AND DISPOSED OF SO AS NOT TO INTERFERE WITH THE PROPER FUNCTION OF THE SWALE.
4. THE SWALE SHALL BE EXCAVATED OR SHAPED TO MEET THE CROSS SECTION SHOWN ABOVE AND SHALL BE FREE OF BANK PROJECTIONS OR OTHER OBSTRUCTIONS THAT MAY IMPAIR THE SWALE'S FUNCTION.
5. NON-WOVEN GEOTEXTILE FABRIC SHALL BE INSTALLED ON THE BOTTOM AND SIDES OF THE TRENCH AS WELL AS BETWEEN THE STONE AND SAND LAYERS.

[illegible]

PRIOR TO ANY EARTH  
DISTURBANCE THE CONTRACTOR  
SHALL CALL IN A TICKET TO  
DIG SAFE NY AND OBTAIN A  
CLEAR TO DIG

IT IS A VIOLATION OF SECTION 7209 OF THE NYS EDUCATION LAW FOR ANY PERSON TO ALTER ANY ITEM ON THIS PLAN IN ANY WAY UNLESS HE/SHE IS ACTING UNDER THE DIRECT SUPERVISION OF A PROFESSIONAL ENGINEER.

CHRISTOPHER D. LONGO, PE  
N.Y.S. LIC. # 095840



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PROJECT  
CAROGA LAKE ARTS  
COLLECTIVE, INC.

1989 STATE HWY 10  
CAROGA LAKE, NY

Title E&SC DETAILS	
Date 06/23/2021	Sheet
Scale N.T.S.	C501
Job# 21028	
Sheet 2 of 2	